

### **CHADWICH LANE QUARRY LTD**



# CONSTRUCTION QUALITY ASSURANCE VALIDATION REPORT

EARTHWORKS CONSTRUCTION FOR BASAL GEOLOGCAL BARRIER PHASE 3

**Section 3** 

REF:ARM/SCM/CLQ/CQA/1.00/2011

Revision 1.00

### CHADWICH LANE QUARRY LANDFILL SITE

### EARTHWORKS CONSTRUCTION FOR BASAL GEOLOGICAL BARRIER BASE PHASE 3 Section 3

# CONSTRUCTION QUALITY ASSURANCE VALIDATION REPORT

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### CHADWICH LANE QUARRY LANDFILL SITE

### **EARTHWORKS CONSTRUCTION**

## BASE PHASE 3 Section 3

# CONSTRUCTION QUALITY ASSURANCE VALIDATION REPORT

### 1. INTRODUCTION

In March 2011, Enviroarm Limited were requested by Chadwich Lane Quarry Ltd to carry out supervision and quality control tests on earthworks construction of an engineered geological barrier on Phase 3 Section 3 at the Chadwich Lane Quarry landfill site.

Initial preparation works commenced during the later part of February and early March with final sand extraction in Phase 3 Section 3, completed in April 2011 down to the 176m AOD.

Mineral liner works on Section 3 commenced on 21<sup>st</sup> March 2011 and were completed on 6<sup>th</sup> May 2011.

During the period and most of May the final sand was extracted from Section 3 down to a depth of 176m AOD.

A final section of sand is now left to remove from the base to be completed during the summer of 2011.

The mineral liner works were carried out by Chadwich Lane Quarry Ltd.

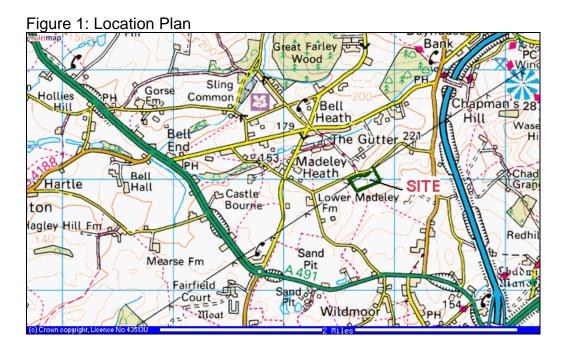
During the works, Enviroarm Limited personnel were in attendance on site on a part time basis to assess and control the sub grade formation, mineral liner construction and to carry out all testing.

The following Construction Quality Plans were used during the construction of the cell:

 Enviroarm Limited Chadwich Lane Quarry Construction Quality Assurance Plan • Enviroarm Limited Source Evaluation of material for use in Geological Barrier at Chadwich Lane Quarry.

### 2. THE SITE

The Chadwich Lane Quarry landfill site is an operational landfill as defined under the Landfill Directive for the receipt of inert waste. Phase 3 Section 3 is the penultimate base section to be engineered in accordance with requirements of the Permit TP3431SG. The former quarry off Chadwich Lane is situated on the south-western fringe of Birmingham at National Grid Reference SO 958 771. It is approximately 3km east of Rubery and 3km north of Catshill.



### 3. **FORMATION**

### 3.1 General

The excavation was in clean virgin Sherwood Sandstone. The side walls remained stable with no tension cracks visible and the base was excavated flat. The overall configuration was in accordance with the Slope Stability Risk Assessment for the site. The sand is well cemented and requires a minimum 30 tonnes excavator to break and remove the sand.

### 3.2 <u>Formation Compaction</u>

Following the excavation of sand from the each section each section the exposed lower sub grade was subject to 8 passes with the excavator tracks all over the surface prior to the placement of the geological barrier.

### 4. GEOLOGICAL BARRIER MINERAL LINER

#### 4.1 General

The geological barrier mineral liner of Phase 3, Section 3 were constructed using re-worked on-site clay bands excavated as part of the quarrying operations which had been selectively dug and stored separately from the sand. The accepted placement specification was a target permeability of 1x10<sup>-7</sup>m/s achieved between a moisture wet of optimum. The minimum clay fraction limit was set at 10% and minimum shear strength of 50kPa, and placed at 5% air voids. The mineral liner was to be placed to a thickness of 1000mm.

#### 4.2 Compaction Testing

The initial Lift 1 was used as the compaction trial on the clay with the results faxed through from the soils laboratory prior to commencement of the next lift on the 21<sup>st</sup> March 2011, to ensure that the compaction had been achieved, and that the use of the hydraulic excavator tracking over the clay would achieve the necessary compaction of the liner.

The excavator was a 30 tonne Doosan DX300 and the lifts were placed at 300mm and compacted to 250mm using the tracks.

The same method of compaction was used for all of the Section 3 works detailed in this report.

#### 4.3 Site Visits and Control Testing

Part time monitoring of the works was carried out during the construction of the engineered low permeability geological barrier mineral liner to check working procedures and to undertake quality control testing to ensure compliance with the approved Construction Quality Assurance Plan. The barrier was divided into two sections, each approximately 35 metres x 30 metres, giving each section a total surface area of 2100m<sup>2</sup> and a placed volume of 2,100m<sup>3</sup>. Initial phases have previously been over tested against the CQA Plan to ensure that the compaction methodology was acceptable and to account for unknown variance in the clay.

The approximate test locations re presented on Drawing CLQSD2.

The clay was placed at natural moisture content as this demonstrated natural moulding parameters from basic on site puddling tests.

On site moisture compliance testing was carried out as a way of Construction Quality Control for the works, using an Ashworth Speedy Moisture Gauge and temperature control for placement was using an Environment Meter.

All testing was undertaken in accordance with BS 1377: 1990 and BS 5930:1999 at frequencies listed in Construction Quality Assurance Plan for Phase 3. In-situ tests using core cutters were undertaken to ensure that the material was placed with less than 5% air voids and at the correct moisture content.

Permeability tests were carried out using on site falling head core tests as per Environment Agency Guidance for inert landfill sites.

The total volume was calculated at 2,100m<sup>3</sup>.

The corroborative core cutters were taken in accordance with BS1377: 1999 and submitted to Ground Investigation and Piling Limited UKAS soils laboratory in Wolverhampton.

Ground Investigation and Piling Limited are UKAS accredited for Density, Moisture Content, Specific Gravity and Atterberg Limits. The UKAS Certificates are presented at Appendix 1 Ref 1897, Issue 010, August 2010.

All testing has been carried out in accordance with BS1377:1990, and reported in accordance with BS1377:1990 UKAS requirements, with the exception of the permeability tests which have been carried out as per Environment Agency Guidance.

Daily log sheets were completed for each working day by the site engineer and are presented in full at Appendix 6.

The excavator was observed traversing each lift with a minimum number of eight passes prior to any testing being carried out. In general the excavator did on average 10 passes on each lift.

### 4.4 <u>Field Test Results</u>

All field samples were taken by the Engineer during each day of works.

All testing carried out with the Speedy Moisture Gauge to control the moisture content of the conditioned material both at the processing area and placed in Section 3 are reported in the daily log sheets presented at Appendix 6.



Plate 1:17% moisture on Ashworth Speedy Moisture Tester

No clay was placed in a freezing condition or when temperatures were below 0°C. Temperature was monitored using an environment meter. The clay was clean and had been stored separately from the sand and had been previously selectively excavated. All of the material used was sourced from on site from the same band of clay used for all previous lining works on Phase 1, 2 and the side wall seal.

### Core Cutter Assessment

Core Cutter tests were carried out on each of the four lifts on the base at a rate of one test per 525m<sup>3</sup>. The testing and pass-fail results are presented at Appendix 2.

The results of the cores on each sheet have been correlated against the upper specific gravity test value obtained on all of the sections. All of the results are presented at Appendix 2. The results from each section have been presented as the laboratory sheets, followed by graphical plots.

All of the plots were above the 5% air voids line.

The holes produced taking the Core Cutters were sealed using additional clay which was tampered into the hole using the core puner and then additional clay placed on top and subject to compaction with the compaction plant with a minimum 8 passes.

### Specific Gravity Tests

The specific gravity or particle density value is presented at Appendix 3.

The test reported value was 2.70, this is similar to previous values of 2.69.

All of the holes produced taking the specific gravity bulk bag tests were sealed using additional clay which was tampered into the hole using the core puner and then additional clay placed on top and subject to compaction with the compaction plant with a minimum 8 passes.

### Atterbrg Limits

One Atterberg test was taken at a frequency of one test in 1000m<sup>3</sup>.

The clay used for the mineral liner would be classified as CM-clays of medium plasticity.

Placement of the clay at and above 17% showed the clay to deform plastically during compaction.

The results are presented at Appendix 3.

The only specifications set for plastic limit and classification criteria was that the plastic limit should not be greater than 65% and a plasticity index less than 10%, and that the test frequency should be not greater than one test per 1000m<sup>3</sup>. Both of these criteria were complied with and exceeded.

No tests results were reported outside the British Standard. The plastic limit, liquid limit and therefore the plasticity index tests were reported correctly in accordance with the British Standard. The clay was placed at a moisture content of between a 16% and 17%. Corroborative bulk tests were taken for Natural Moisture Content with the Atterberg Limit determinations.

Control of moisture was carried out throughout the works using an Ashworth Speedy Moisture Tester.

The holes produced taking the Atterberg Limits and Natural Moisture Content bulk bag test were sealed using additional clay which was tampered into the hole using the core puner and then additional clay placed on top and subject to compaction with the compaction plant with a minimum 8 passes.

### Permeability Tests

The permeability tests were carried out in accordance with the Environment Agency Guidance. Permeability test records are presented at Appendix 4.

All of the holes produced taking the Permeability cores were sealed using additional clay which was tampered into the hole using the core puner and then additional clay placed on top and subject to compaction with the compaction plant with a minimum 8 passes.



Plate 2: Core permeability falling head test on Phase 3

### **Shear Strength Tests**

20 shear strength tests were undertaken at a rate of at least one test per 105m<sup>3</sup> using an EDECO/Pilcon hand shear vane at random locations on the base and sides of each lift, within each section of geological barrier mineral liner placed, and are presented on summary sheets Appendix 5.

All of the results are well in excess of 50kPa.

The test locations together with the layer on which each test was taken are shown the daily log sheets at Appendix 6.

Titale 3. Hand shear varie result in excess of rooks a

Plate 3: Hand shear vane result in excess of 100kPa

### 4.5 Assessment of Earthworks Construction

The Section 3 of Phase 3 are presented on Drawing CLQP3.1.

### Section 1

Placement of the clay was carried out in four lifts between 21<sup>st</sup> March and 6<sup>th</sup> May in two phases. The plastic limit was reported at 17% and had a natural moisture content of 16% to 17%. The permeability falling head test caaied out in 21<sup>st</sup> March still had lost no water on 23 March.

### Section 2

Section 2 moisture values were at 17%. The cores did not loose any water from 29<sup>th</sup> April to 2<sup>nd</sup> May. The Phase 3 Section 2 basal lining works were completed on the 6<sup>th</sup> May 2011.

### General

All moisture controls on site were by use of the Ashworth Speedy Moisture Gauge which had been calibrated on site against core cutters. The moisture gauge readings were found to be within 0.2% of laboratory tests. In addition the feel of the clay was easy to determine the plasticity.

### 4.6 Non-Conformance

No non-conformances were reported during the placement of the geological barrier.

### 5. CONCLUSIONS

The procedures carried out on site were in accordance with the Construction Quality Assurance Plan with appropriate layer thickness and the number of passes of the compaction plant. All materials incorporated within the earthworks were therefore placed satisfactorily.

The results of the permeability tests on falling head cores showed the geological barrier to achieve the standard of 1 x 10 7m/s or less and the core cutters show less than 5% air voids in the barrier.



Plate 4: View of completed lining works for Phase 3

The mineral liner was over 1 metre in thickness as proved by physical measurements by the engineer on site.

The testing carried out on the geological barrier mineral liner both by independent laboratory testing at UKAS accredited soils laboratories and field tests exceeded the required test frequencies in the agreed CQA Plan and showed that the geological barrier had achieved and exceeded the required minimum standards on all testing.

Detailed and full comprehensive engineers daily logs have recorded the entire construction and testing carried out.

In conclusion all of the materials have been tested to and above the required frequency and all of the materials have exceeded the minimum standard requirements.

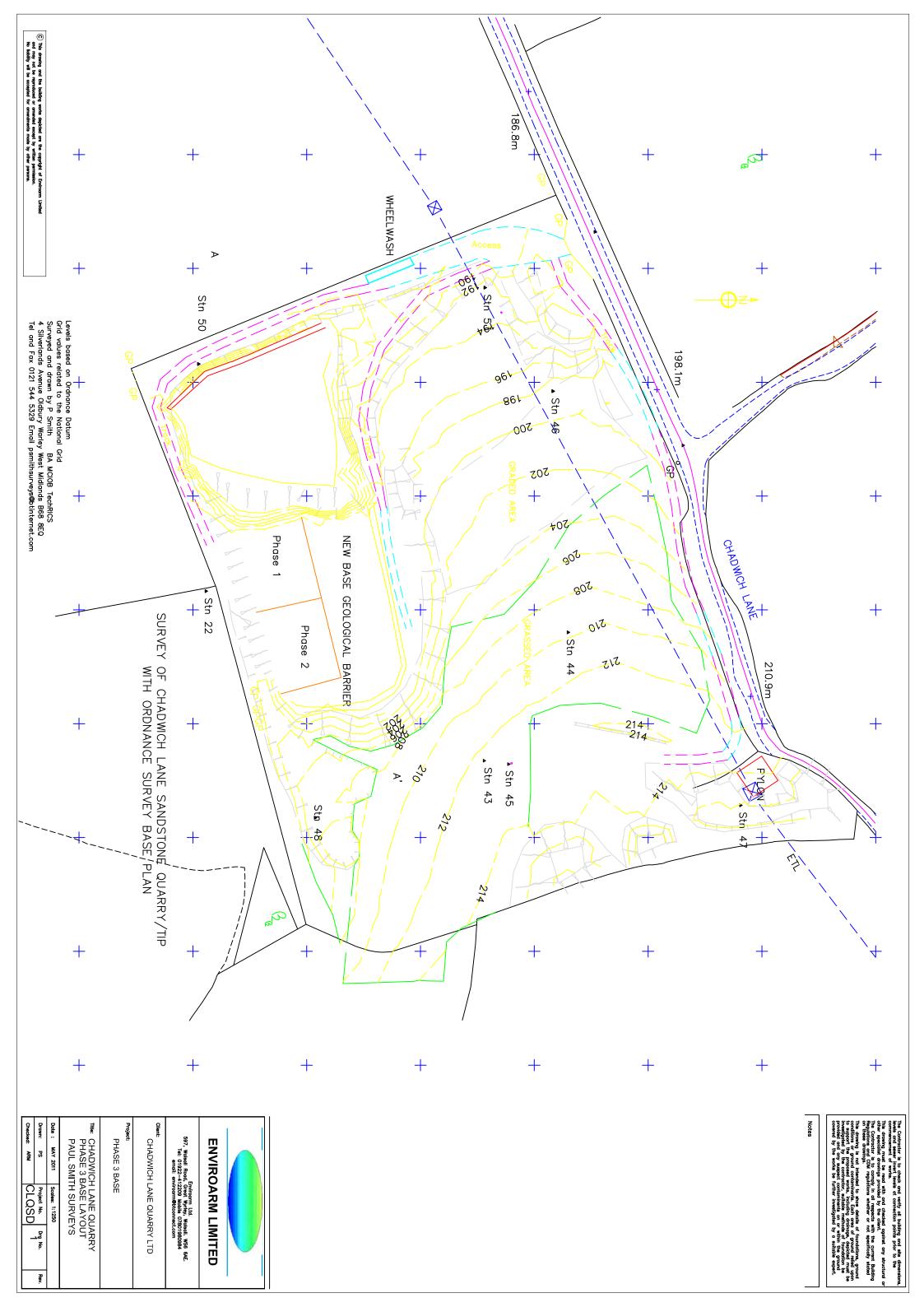
The Phase 3 Section 3 is therefore acceptable for the receipt of inert waste as it has been constructed in accordance with the approved Construction Quality Assurance Plan.

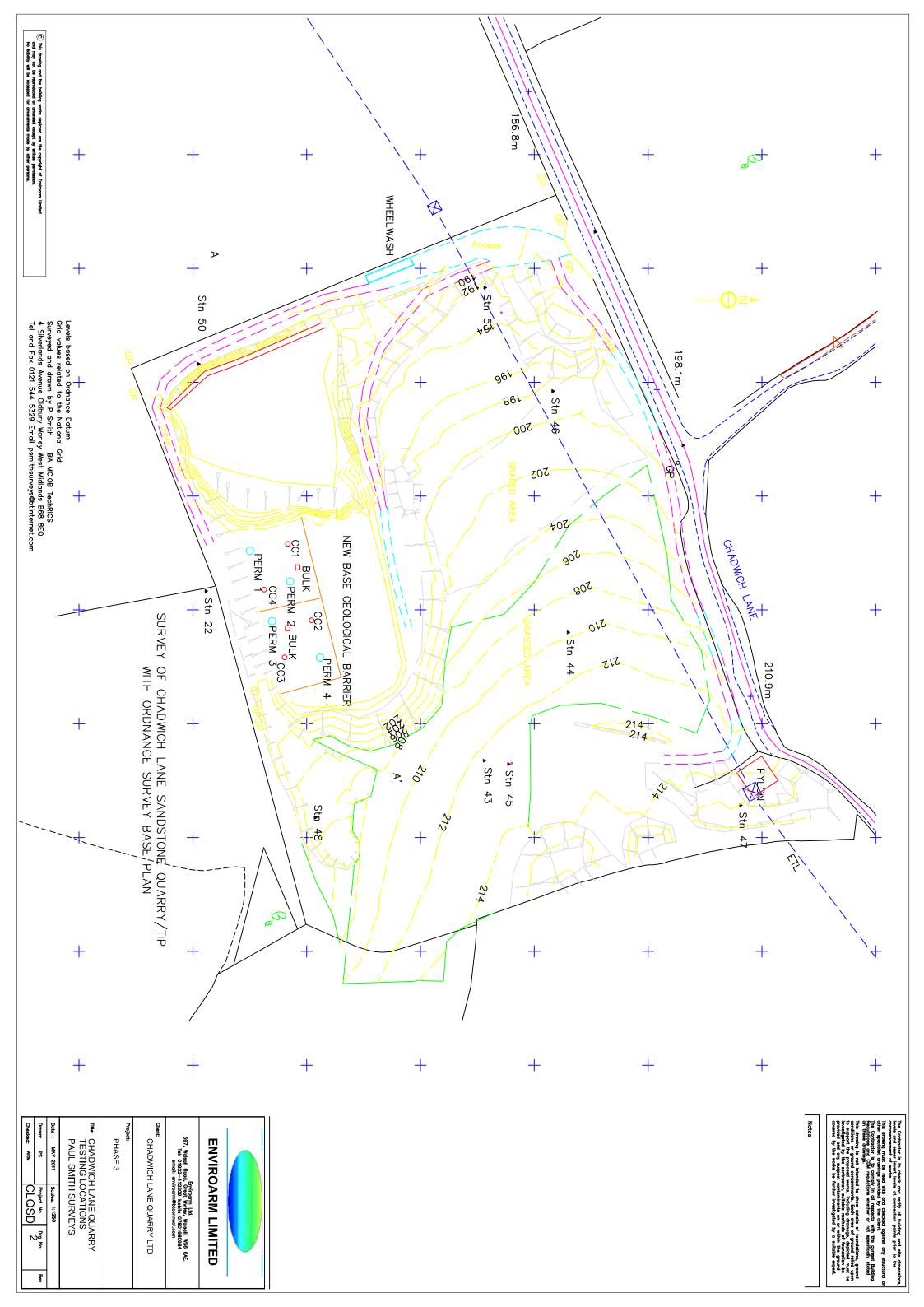
Report prepared by:

A.R.Morris

Date 29<sup>th</sup> May 2011

# **DRAWINGS**





# **APPENDIX 1:**

# UKAS Certificate for Soils Laboratory

### **Schedule of Accreditation**

issued by

### **United Kingdom Accreditation Service**

21 - 47 High Street, Feltham, Middlesex, TW13 4UN, UK



Accredited to ISO/IEC 17025:2005

### **Ground Investigation and Piling Ltd**

Issue No: 010 Issue date: 26 August 2010

Devonshire House Ettingshall Road Wolverhampton West Midlands WV2 2JT

Contact: Mr. P. Smart
Tel: +44 (0)1902-459558
Fax: +44 (0)1902-459085
E-Mail: info@gipuk.com
Website: www.gipuk.com

### Testing performed at the above address only

#### **DETAIL OF ACCREDITATION**

Materials/Products tested	Type of test/Properties measured/Range of measurement	Standard specifications/ Equipment/Techniques used
SOILS for civil engineering purposes	Moisture content - oven drying method	BS 1377:Part 2:1990
	Liquid limit - cone penetrometer	BS 1377:Part 2:1990
	Liquid limit - cone penetrometer - one point	BS 1377:Part 2:1990
	Plastic limit	BS 1377:Part 2:1990
	Plasticity index and liquidity index	BS 1377:Part 2:1990
	Particle size distribution - dry sieving	BS 1377:Part 2:1990
	Particle size distribution - sedimentation - pipette method	BS 1377:Part 2:1990
	Particle density - gas jar	BS 1377:Part 2:1990
	Particle size distribution - wet sieving	BS 1377:Part 2:1990
	Sulphate content of soil and ground water - gravimetric method	BS 1377:Part 3:1990
	pH value	BS 1377:Part 3:1990
	California Bearing Ratio (CBR) (loads from 0.2 to 20 kN)	BS 1377:Part 4:1990



### **Schedule of Accreditation** issued by United Kingdom Accreditation Service 21 - 47 High Street, Feltham, Middlesex, TW13 4UN, UK

### **Ground Investigation and Piling Ltd**

**Issue No:** 010 Issue date: 26 August 2010

### Testing performed at main address only

Materials/Products tested	Type of test/Properties measured/Range of measurement	Standard specifications/ Equipment/Techniques used
SOILS for civil engineering purposes	Dry density/moisture content relationship (2.5 kg rammer)	BS 1377:Part 4:1990
	Dry density/moisture content relationship (4.5 kg rammer)	BS 1377:Part 4:1990
	Density of specimens sampled using the core cutter method	BS 1377:Part 9:1990
	END	

# **APPENDIX 2:**

# Core Cutter test results and Graphs

### LABORATORY REPORT FOR IN-SITU DENSITY TESTS

Contract:	Chadwich	Lane			Custom		m Limited, Isall Road,		A	
						Great W				Ltd. Ground Investigation & Piling Limited
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Date Received:-		23.03		sued:- 24.0	03.11	WS6 6A	Æ.	_	Phone 01902 4595	58, Fax 01902 459085, email lab@gipuk.com
SAMPL	E DETAILS	5	TEST DATE							
SAMPLE	DEPTH	TEST	BATTE	W	Bulk Density	Dry Density	Percentage Compaction	SAMPLE DESCRI	PTION	REMARKS
No.	m	TYPE		%	Mg1m <sup>3</sup>	Mg1m³	%			
Base Lift 1	CC 1.1	CC	21.03.11	16	2.10	1.80		Firm red brown slightly sandy CLAY. Gravel is weak mudsto		
Base Lift 4	CC 4.1	CC	23.03.11	17	2.14	1.83		As above.		
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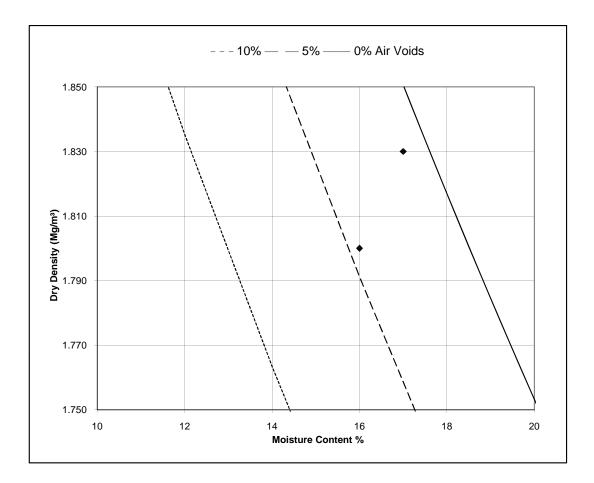
### LABORATORY REPORT FOR IN-SITU DENSITY TESTS

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						Great W				Ltd Ground Investigation & Piling Limited
Job No:-	L/7603		Page No	o:- 1.	of 1	Nr.Wals Stafford				shire House, Ettingshall Road, olverhampton. WV2 2JT
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No.	m	TYPE		%	Mg1m³	Mg1m³	%			
Base Lift 2	CC 2.1	CC	04.05.11	17	2.12	1.81		Firm red brown slightly sandy CLAY. Gravel is weak mudsto		
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Base Lift 3	CC 3.1	CC	04.05.11	17	2.13	1.82		As above.		
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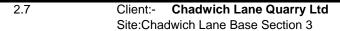
Client:- Chadwich Lane Quarry Ltd Site:Chadwich Lane Base Section 3

Graph 1 Base

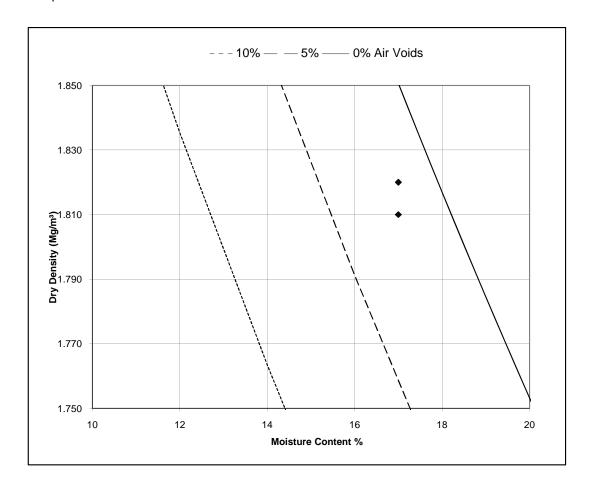


MC	Density
16	1.800
17	1.830
0	0.000
0	0.000
0	0.000
0	0.000
0	0.000
0	0.000
0	0.000
0	0.000
0	0.000

Particle density 2.700



Graph 1 Base



MC	Density
17	1.810
17	1.820
0	0.000
0	0.000
0	0.000
0	0.000
0	0.000
0	0.000
0	0.000
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0	0.000

Particle density 2.700

### **APPENDIX 3:**

# **Specific Gravity and Classification Tests**

### LABORATORY REPORT FOR INDEX PROPERTY AND CHEMICAL TESTING

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			DATE			ex Prope		1				_		BS S		SAMPLE DESCRIPTION	COMMENTS
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No.	m			%	%	%	%	Mg/m³	%	g/L	Value	%	%	mm	mm		
Base	Lift 2	В	23.03.11	16	39	17	22	2.70							99	Firm red brown slightly sandy slightly gravelly	
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	Disturbed		W	Moisture		W%	BS1377:F	Part 2:1990		Total SO <sub>4</sub>		77:Part 3:					
В	Bulk distu		WL	Liquid lin	nit	WL	BS1377:F	Part 2:1990	):4.4	Sol SO <sub>4</sub>	BS13	77:Part 3:	1990:5.5				
U	Undisturb	ed	WP	Plastic li		WP	BS1377:F	Part 2:1990	0:5.3	pH Value		77:Part 3:				(	
S	SPT split	spoon	lР	Index pro		IР		Part 2:1990								E IIVAS =	
W	Ground w	ater	PD	Particle		PD		Part 3:1990		Approved	l signatory	:-				U KAS TESTING	
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				Organic				Part 3:1990	):3							Tests marked * are not UKAS	
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### LABORATORY REPORT FOR INDEX PROPERTY AND CHEMICAL TESTING

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											Great W					Ground Investigation & Piling Limited	
					_						Nr.Wals					Devonshire House, Ettingshall Road,	
Job No:	-		<b>∐</b> 7603		Page No	D:-	1 (	of 1			Stafford	shire.				Wolverhampton. WV2 2JT	
Date Re			04.05	.11	Date Iss			5.11			WS6 6A					Phone 01902 459558, Fax 01902 459085, Email lab@gipuk.com	
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No.	m	TTPE		%	%	%	%	(Gas Jar) Mg1m3	30₄ %	SO₄ g1L	Value	%	%	2.00 mm	0.425 mm		
Base	Lift 4	В	04.05.11	18	40	17	23	2.70	70	giL	value	70	70	111111		Firm red brown slightly sandy slightly gravelly	
Base	Liit		04.00.11	10	10	.,	20	2.70							- 55	CLAY. Gravel is weak mudstone.	
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												l					
	Sample typ	е	Test a	abbreviati	ions		•		Te	est method	ls - Unless	otherwise	stated.		•	ch	
	Disturbed			Moisture	Content	W%	BS1377:F	Part 2:1990	:3.2	Total SO <sub>4</sub>	BS13	77:Part 3:	1990:5.5			_ 👮 _	
В	Bulk distu			Liquid lim		WL		Part 2:1990		Sol SO <sub>4</sub>		77:Part 3:					
U	Undisturb			Plastic lir				Part 2:1990		pH Value	BS13	77:Part 3:	1990:9			[(84)]	
	SPT split	-		ndex prop	•	lp 		Part 2:1990								- UKAS -	
W	Ground wa	ater		Particle D	•	PD		Part 3:1990		Approved	signatory	:-				U.K.A.S TESTING	
Т	Tub			Loss on i				Part 3:1990								1897	
	0=:-	iono anali		Organic				Part 3:1990	J.3	ĺ				A 1	Pan	Tests marked * are not UKAS accredited.	
Th:a			interpretation be reproduced			•			tony	Tony Boo	roo Lobo	ratory Dira	otor		,	The reported results relate only to samples received.  # = Sample mass smaller than BS1377 requirements	
inis	iesi report	snan not D	e reproduced	і ехсері ІІ	ı ıuli withot	n written a	рргочаг Бу	пе врога	wy.	rony Pea	iice, Labo	ratory Dire	JUI	•••		# = Sample mass smaller than 551377 requirements	s.

# APPENDIX 4: Permeability Tests

### CHADWICH LANE QUARRY PERMEABILITY TESTS



Phase 3 Section 1: Falling Head Permeability 21/03/2011



Phase 3 Section 1: Falling Head Permeability 23/03/2011

### CHADWICH LANE QUARRY PERMEABILITY TESTS



Permeability Test Phase 3 Section 2 4<sup>th</sup> May 2011



Permeability Test Phase 3 Section 2 6<sup>th</sup> May 2011

# APPENDIX 5: Shear Vane Tests

### **Shear Vane Test Results:**

Date	Test Location:	Passes:	Shear Vane Results kPa
21.03.2011	Base P3.1	Lift 1	102,106
	Base P3.1	Lift 2	98, 106
23.03.2011	Base P3.1	Lift 3	100,94
	Base P3.1	Lift 4	94,112
28.04.2011	Base 3.2	Lift 1	87,92
04.05.2011	Base 3.2	Lift 2	94,102,95
	Base 3.2	Lift 3	104,84,93
06.05.2011	Base 3.2	Lift 4	101,97,96,112,106
AVERAGE			99

### **APPENDIX 6:**

# **Enviroarm CQA Engineers Daily Logs**



		T =: 0''	Time Off Site:
Site:	Date of Visit:	Time on Site:	Time Oil Site.
CHADWICH	21/3/2011	10:45	14:20
LANE	21/3/2011		
PHASE 3			
	0.01	Equipment:	
Personnel/Staff:	Site Conditions/Weather:	DOOSAN E	Dx 200
A.R. MOERIS	CLOUD, DR.7.	DOCS! I. P	
CLQ			
	Rain off: Yes No		
Operations	Action: COMPACTION T	RIPO LIFT	162
Inspected:			,
ISHSE			
Comments on Work	manship:	to 250	mm
300 mm	manship: tracked down	avater.	
April 106	sper Cours		
A25 com	acting day.		
In atmestiona/Informati	tion Issued or Required.		~
Same da	1 source ba	nd as h	revous.
July Clery	1 2000	,	
	$\circ$		
Progress Report (use	e additional sheets if necessary)		
Core taker	DEILY 0102 0106	LIFT	2.
Chamlin	VI 102 0106	698	· 106
2 rocks   FIN	DEIH STORE		
GNZ.			

Signature: A, R. Morriso



Site:	Date of Visit:	Time on Site:	Time Off Site:
		Timo on one.	
CHADWICH	23 3 2011	12:00	15:00
LANE		. 2 00	
Personnel/Staff:	Site Conditions/Weather:	Equipment:	
	61007 6001	DOOSAN	Dx300
A.R. MORRIS	WARN	AZS VOLV	
CLQ,		11/2	
	Rain off: Yes/No		
Operations Inspected:	LIFTS 3 Y	A con	MPLETED
BASE P3			~
SECTION 1.			
Comments on Workr	nanchin:		
	ompacted lif	TS	
2)0,0	J	,	
		,	
Instructions/Informat	ion Issued or Required:	ift 3	· · · · · · · ·
	_	3	LIFT 4
m oby	KTLIFTZ,		
	}	000	994
OPERM L	t.)	94	0112
~	cel.		
Progress Report (use	additional sheets if necessary)		
IMETRE	COMPLETED -	- SECTION	)
1110	T IL RULK C	occ ust	2 PRIOR
CC - LIF	COMPLETED - T 4 BULK O MENT OF LIF	T 3	
TO PLACE		3	

Signature: A.R. Morro



9:45 12:00
lipment:
0054N DX300 BT D6 BUO A25
SE STABILITY WORK
LATERS WITH CAT ( EGRADING LOPE AREA

Signature: A.R. Morris



Site:	Date of Visit:	Time on Site:	Time Off Site:
CHADWICH CAME QUARREY.	4/5/2011.	10:15 -	11:40-
Personnel/Staff: A.R. MOPRIS	Site Conditions/Weather:	Equipment:  DODSAN C  VOLVO A 25  CAT D6.	
Operations Inspected: PHISE 3	Action:	3 PLACE!	>,
SECTION 2	la im-		
Comments on Workn 250 mm Lh	TERS TRACKED	IN WITH C	AT D6.
SHEPPS 2 51. 024 51. 02	ion Issued or Required:  LIFT 3,	3, PERMAN	UFT 4
Progress Report (use CORES SEN SHEARS OF	additional sheets if necessary)  TO GIP-  K.	CC LZ PERMOCC LS	

Signature: A.R. Morro



Site:	Date of Visit:	Time on Site:	Time Off Site:		
CHADWICH	6/5/2011	11:00	12:00		
Personnel/Staff:  A. P. MORRIS  B. WOOD-	Site Conditions/Weather:	Equipment:	Dx300		
Operations Inspected: PHASE 3 LIFT 4	Action: FINAL WORKS	5 ON BA	SE SECTION 3		
Comments on Workmanship:  PERM OL,  SHEARS OK  FINAL SECTION OF LIFT A TRACKED IN.  Instructions/Information Issued or Required:  101  96  98014.					
PERM, O HO	additional sheets if necessary)	918.			

Signature: 12. R. Morro