ENERGY AND CLIMATE CHANGE ENVIRONMENT AND SUSTAINABILITY INFRASTRUCTURE AND UTILITIES LAND AND PROPERTY MINING AND MINERAL PROCESSING MINERAL ESTATES WASTE RESOURCE MANAGEMENT

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COVENTRY CITY COUNCIL

SHERBOURNE RESOURCE PARK

FLOOD RISK ASSESSMENT

AUGUST 2020





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AUGUST 2020

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1 INTRODUCTION

1.1 General

- 1.1.1 Wardell Armstrong LLP (WA) has been commissioned by Coventry City Council to prepare a Flood Risk Assessment (FRA) for the proposed development of a Materials Recycling Facility (MRF) on land off Shortley Road, Coventry, West Midlands.
- 1.1.2 This report sets out the findings of the FRA required by the Local Planning Authority to support the planning application for development on the site. The assessment has been carried out in accordance with the guidance set out in National Planning Policy Framework (NPPF).

1.2 Methodology

- 1.2.1 The methodology for this FRA has comprised a desktop study, supplemented by liaison with Coventry City Council, the Environment Agency and Severn Trent Water.
- 1.2.2 Reference has been made to relevant plans and documents including:
 - Coventry City Council (December 2015) Level 1 and Level 2 Strategic Flood Risk Assessment;
 - Coventry City Council (June 2017) Preliminary Flood Risk Assessment Second Cycle Final Report;
 - Coventry City Council (2019) Flood Risk Management and Drainage Planning Standing Advice; and
 - Severn Trent Water Sewer Record Plans.



2 FLOOD RISK AND PLANNING POLICY

2.1 National Planning Policy

- 2.1.1 The NPPF and the accompanying Planning Practice Guidance (PPG) aim to ensure that flood risk is taken into consideration at all stages of the planning process and advocates the use of a risk-based 'Sequential Test' to preferentially locate development in areas with a low risk of flooding. Where development is necessary in high risk areas, the NPPF aims to ensure that the development is safe without increasing flood risk through the application of the Exception Test.
- 2.1.2 The PPG, defines the levels of flood risk within England as follows:
 - Flood Zone 1 Low Probability Land having less than a 1 in 1,000 annual probability of river or sea flooding.
 - Flood Zone 2 Medium Probability Land having between a 1 in 100 and 1 in 1,000 annual probability of river flooding; or between a 1 in 200 and 1 in 1,000 annual probability of sea flooding.
 - Flood Zone 3a High Probability Land having a 1 in 100 or greater annual probability of river flooding; or having a 1 in 200 or greater annual probability of sea flooding.
 - Flood Zone 3b Functional Floodplain Land where water has to flow or be stored in times of flood.
- 2.1.3 The PPG states that a site-specific FRA is required for all new development proposals located in Flood Zones 2 and 3, and for any proposal of 1 hectare or greater regardless of its flood zone classification. The flood zones as described above are shown on the Environment Agency's Flood Map for Planning, available online.
- 2.1.4 Table 2 of the PPG classifies development types based on their vulnerability to flooding, ranging from 'Essential Infrastructure' which has to be operational in times of flood, through 'Highly Vulnerable' (eg emergency service stations), 'More Vulnerable' (eg residential dwellings and establishments), 'Less Vulnerable' (eg offices/retail), to 'Water Compatible' development (eg open space, docks, marinas and wharves).
- 2.1.5 Table 3 of the PPG indicates which 'vulnerability classes' are acceptable in each of the Flood Zones, and when the Exception Test should be applied. This is reproduced as Table 1 below.



| Table 1: Flood Risk Vulnerability and Flood Zone 'Compatibility' | | | | | | | |
|--|-----------------------------|----------------------|-------------------------------------|--------------|---------------------|--|--|
| Flood Zone | Essential Infrastructure | Highly Vulnerable | Highly More ulnerable Vulnerable | | Water Compatible | | |
| 1 (>0.1%) | ✓ | ✓ | √ | √ | · · | | |
| 2 (0.1 – 0.5%) | ✓ | Exception Test | \checkmark | \checkmark | √ | | |
| 3a (>0.5%) | Exception Test | | Exception Test | \checkmark | √ | | |
| 3b (>5%) | Exception Test | | | | √ | | |

2.2 Application of the Sequential and Exception Test

- 2.2.1 The site is shown on the Environment Agency's Flood Map for Planning to be located wholly within Flood Zone 1 (see Figure 1 and Figure 2).
- 2.2.2 The Sequential Test, as set out in the NPPG, aims to steer developments to areas with the lowest risk of flooding (ie Flood Zone 1 where possible). As the site is located wholly within Flood Zone 1, based on the guidance in Table 3 of the NPPG, the Sequential Test is deemed to have been passed.
- 2.2.3 The Exception Test, detailed in paragraph 159 of the NPPF, should be applied only after the Sequential Test has been applied and in circumstances when 'More Vulnerable' development and 'Essential Infrastructure' cannot be located within Flood Zones 1 or 2, or 'Highly Vulnerable' development cannot be located within Flood Zone 1.
- 2.2.4 The proposed industrial land use is classified as 'Less Vulnerable' development in Table2 of the NPPG. Table 1 above shows that a 'Less Vulnerable' development is appropriate within Flood Zones 1 and 2 and, consequently, it is not necessary to apply the Exception Test.





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Figure 1. Environment Agency Flood Map for Planning





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Figure 2. Environment Agency Flood Map for Planning – Site Entrance



3 SITE SETTING

3.1 Site Description and Location

3.1.1 A summary of the site and its characteristics is provided in Table 2.

| Table 2: Site Location Summary | | | | |
|--------------------------------|--|--|--|--|
| Site Name | Coventry MRF | | | |
| Site Address | Land off London Road, Coventry, West Midlands, CV3 4AR | | | |
| Site Area (ha) | 4.45ha | | | |
| National Grid Reference | 434812, 277541 | | | |
| Existing Land Use | Former allotments | | | |
| Proposed Land Use | Materials Recovery Facility | | | |
| Local Planning Authority | Coventry City Council | | | |
| Sewer Undertaker | Severn Trent Water | | | |

- 3.1.2 The site is located off London Road (A4082) in Coventry, approximately 1.25km to the south-east of the city centre. The National Grid Reference for the approximate centre of the site is 434812, 277541 and the nearest postcode is CV3 4AR. The location of the site is shown on Drawing No CA11485-021 *'Site Location Plan'*.
- 3.1.3 The site can be divided into two areas. The main, and northern, area of the development site is generally rectangular in shape and comprises a number of disused allotment plots. This area is bounded to the north and south by other disused allotments; to the west by woodland and the Coventry Household Waste Recycling and Re-Use Centre for an adjacent Energy from Waste Facility; and to the east by occupied allotments.
- 3.1.4 The small former allotment plots within the site area have been disused for a number of years and are now heavily overgrown by rough vegetation, trees and shrubs. A number of pathways cross the site area and there is also the presence of a number of old building foundations.
- 3.1.5 The remaining southern part of the site comprises a 300m section of access road directly off London Road which is located over areas of hardstanding within Coventry City Council's existing Whitley Depot. This section of the development site is bounded to the east by light industrial units (Whitley Depot) and to the west by disused allotments and woodland, with the River Sherbourne located further beyond.
- 3.1.6 The topography of the main site is shown on Drawing No CA11485-025 *'Site Topography'*. Ground levels within the main part of the site area fall north-westwards,



westwards and south-westwards from a maximum elevation of approximately 89.15mAOD on the eastern site boundary to a minimum elevation of approximately 72.90mAOD in the south-western corner. Ground levels fall most steeply in the north-western and south-western area of the site, with typical gradients of approximately 1 in 6 and 1 in 9 respectively. Slopes in central and eastern parts of the main site area are less steep with typical gradients of approximately 1 in 15.

3.1.7 Ground levels along the route of the access road fall in a southerly direction from a maximum of approximately 79.49mAOD at the northern extent, where it meets the main part of the site, to approximately 72mAOD at its junction with London Road in the south. The topography at the junction with London Road is shown on Drawing No CA11486-038 'London Road Entrance Topography'.

3.2 Existing Watercourses and Waterbodies

- 3.2.1 There are no watercourses or bodies of water within the site area. The closest watercourse to the site is the River Sherbourne, a Main River, located approximately 20m to the west of the site boundary at its closest point. The River Sherbourne flows southwards to ultimately join the River Sher, approximately 2km to the south of the site.
- 3.2.2 The River Sherbourne, whilst located close to the site, is situated several metres lower than the site entrance with London Road (A4082). As shown on Drawing No CA11485-038 *'London Road Entrance Topography'*, based on LIDAR data, the banks of the watercourse are situated at an elevation of approximately 67mAOD in comparison to an elevation of approximately 72mAOD at the site entrance.
- 3.2.3 There are two small ponds located to the east of the site, approximately 170m and 240m from the site boundary. Both ponds appear to be isolated waterbodies with no obvious inlets or outlets.
- 3.2.4 There are no other watercourses or waterbodies identified in the vicinity of the site.

3.3 Flood Risk Setting

3.3.1 As shown on the Flood Map for Planning in Figures 1 and 2 above, the site is located wholly within Flood Zone 1 with an annual probability of flooding of less than 1 in 1000 (ie a probability of less than 0.1%). Areas of Flood Zone 3 associated with the River Sherbourne are located immediately to the south and west of the site, with an annual probability of flooding of greater than 1 in 100 (ie a probability of greater than 1 in 100). There are no Ordinary Watercourses identified in the vicinity of the site which could affect this classification.



3.4 Existing Drainage

- 3.4.1 Public sewer records, obtained from Severn Trent Water (see Appendix 1), show that a large diameter public combined gravity sewer crosses the site, flowing in a southeasterly direction. The brick egg-shaped sewer, 1.52m by 1.02m in size, is approximately 3m deep at the upstream manhole (ref: SP3477701) located to the north-west of the site and 7m deep at the downstream manhole (ref: SP34778301) located close to the south-eastern boundary of the site within Whitley Depot. The sewer is also shown to be approximately 15m deep in central areas of the site (ref: SP34778501).
- 3.4.2 As stated in the Severn Trent Water Developer Enquiry response (see Appendix 2), the status of the sewer is unknown as sections of the sewer upstream have been abandoned and the incoming flows diverted. A CCTV survey was undertaken on the sewer in April 2019 by OnSite (see report in Appendix 3) and recorded that the sewer upstream of manhole SP34778301 was dry with no evidence of any incoming flows from upstream manhole SP34777701. As the majority of the site area has been used as allotments for over 90 years, it is assumed that there are no private drainage networks within the site area discharging to the public sewer. It is, therefore, unclear why the section of sewer crossing the site has not also been abandoned as there appears to be no incoming flows from either upstream of the site or within the site itself. The status of the sewer is to be investigated as part of future works.
- 3.4.3 The light industrial estate to the south of the main part of the site is served by a private drainage network as shown on Drawing No 2/15816 (*'Whitley Depot, London Road, Coventry*) and 2/14014 *'Whitley Depot London Road Main Yard Plan 2'*). The surface water drainage present within the existing access road falls within the ownership boundary of the proposed development. Gullies within the access road and the wider industrial estate discharge to one of three surface water drainage networks with outfalls to the River Sherbourne to the south and south-west of the site. Private combined sewer networks serving the industrial estate flow under the access road and discharge to the public combined sewer.
- 3.4.4 The existing site drainage has been collated and is shown on Drawing No. CA11485-034 'Existing Drainage Layout'.



3.5 Ground Conditions

- 3.5.1 The online British Geological Survey (BGS) 'Geology of Britain Viewer'¹ shows the majority of the site to be underlain by sandstone bedrock of the Keresley Member. Bedrock of the Helsby Sandstone Formation is present within eastern parts of the main site. Both are classified by the Environment Agency as Principal Aquifers, defined as 'layers of rock or drift deposits that have high intergranular and/or fracture permeability meaning they usually provide a high level of water storage. They may support water supply and/or river base flow on a strategic scale'.
- 3.5.2 There are no superficial deposits shown to be present within the site boundary. Superficial alluvium deposits are located immediately to the west of the site. These are classified as a Secondary A Aquifer by the Environment Agency, defined as *'permeable layers capable of supporting water supplies at a local rather than strategic scale, and in some cases forming an important source of base flow to rivers'*. Clay deposits of the Bosworth Clay Member are shown to be located immediately to the east of the site. These are classified as Unproductive Strata by the Environment Agency, defined as *'rock layers or drift deposits with low permeability that have negligible significance for water supply or river base flow'.*
- 3.5.3 A series of boreholes and trial pits were undertaken within the site area in 2009 as part of an earlier site investigation. Made Ground, consisting of reworked superficial deposits mixed with brick, glass and wood, was encountered to depths of between 0.2m and 2m in northern and eastern areas of the site. Superficial clay deposits were encountered in several boreholes in northern and central areas of the site at elevations of between 88.2mAOD and 68.9mAOD. The upper layer of the sandstone bedrock was encountered at elevations of between 83.5mAOD and 74.8mAOD.
- 3.5.4 The site is shown to be located within the Total Catchment of a groundwater Source Protection Zone, defined as *'the area around a source within which all groundwater recharge is presumed to be discharged at the source'*.

¹ BRITISH GEOLOGICAL SURVEY 'Geology of Britain Viewer' Available at: http://mapapps.bgs.ac.uk/geologyofbritain/home.html



4 DEVELOPMENT PROPOSALS

4.1 Description of the Proposed Development

- 4.1.1 It is proposed to redevelop the site for use as a Materials Recycling Facility as shown on Drawing No CA11485-057 '*Proposed Layout Option 8*'. The main site area will consist of a large processing building with an approximate floor space of 1.4 ha housing a reception hall, processing hall and outfeed hall. A 0.4ha office building will be situated to the south of the main building, with an odour treatment plant, pumphouse and firewater treatment tank located close to the north-western corner of the site. Buildings at the site will be surrounded by areas of hardstanding and car parking. An access road will be constructed over areas of hardstanding within the Whitley Road depot to extend between the new development and the existing access road to the depot off London Road.
- 4.1.2 As part of the proposed development, existing ground levels will be modified to provide flat areas on which the buildings will be situated. The finished floor level of the process hall will be in the order of 1.5m higher than the adjacent reception hall and outfeed hall to the north-west and south and there will be a step between each building. Ground levels within the service yard area will also be modified to give them an even fall.
- 4.1.3 The most significant modifications to the existing ground levels will be in northern areas of the site. Ground levels in the north-eastern corner will be reduced by over 5m to provide a finished floor level of approximately 84mAOD within the process hall. Ground levels in the north-western corner will be increased by up to 8m to provide a level ground on which to situate the odour treatment plant, pumphouse and firewater treatment tank.



5 FLOOD RISK

5.1 Flood Risk to the Development

- 5.1.1 The main sources of flooding identified within the NPPF are rivers, tidal waters and the sea, surface water, groundwater, sewers and drains, and artificial sources such as canals and reservoirs.
- 5.1.2 The presence of a potential flooding source does not necessarily translate into a high risk of flooding. Table 3 below summarises the potential flood sources and the related flood risk posed to the site.

| Table 3: Sources of Flood Risk | | | | | |
|--------------------------------|----------------|----------------|--|--|--|
| Flood Source | Presence | Potential Risk | Description | | |
| | at Site | at Site | Description | | |
| | | | Located wholly within Flood Zone 1. Areas | | |
| Rivers | V | Varia | of Flood Zone 2/3 are approximately 5m | | |
| (fluvial flooding) | Ŷ | very Low | lower than ground levels at the site as a | | |
| | | | minimum. | | |
| Tidal | N | n/a | n/a | | |
| Groundwater | Y | Very Low | Groundwater at sufficient depth | | |
| Surface Water | | | Main site at Very Low risk, small area of | | |
| | v | Vonulow | access route at Low risk, approximately 3m | | |
| rioounig | Ť | Very LOW | lower than ground levels within the main | | |
| (pluvial flooding) | vial flooding) | | site. | | |
| Sower | v | Vonulow | Public sewer crossing the main site | | |
| Jewei | ſ | Very LOW | between 3m to 14m deep. | | |
| Artificial | N | n/a | n/a | | |

Historical Flooding Incidents

- 5.1.3 The 'Historical Fluvial Flooding Records' figure in the Coventry City Council Strategic Flood Risk Assessment (SFRA)² shows a fluvial flooding incident in the vicinity of the River Sherbourne approximately 100m to the south of the site adjacent to the A4114 road. Due to the scale of the map the exact location and extent of flooding cannot be accurately confirmed. However, as ground levels adjacent to the watercourse are approximately 5m lower than ground levels at the site, it is considered that the site was unaffected by this flooding event.
- 5.1.4 The 'Historic Flood Events from Local Sources of Flooding' figure in the Coventry City Council Preliminary Flood Risk Assessment (PFRA)³ shows two fluvial flooding

² COVENTRY CITY COUNCIL (2015) *Level 1 and 2 Strategic Flood Risk Assessment*

³ COVENTRY CITY COUNCIL (2017) Preliminary Flood Risk Assessment – Second Cycle Final Report CA11485/0005/FINAL



incidents, recorded between 1990 and 2011, adjacent to the River Sherbourne located approximately 10m and 100m to the south of the site. As with the figure in the SFRA, an accurate location and extent of the flooding incident cannot be confirmed, however, as ground levels are approximately 5m lower than ground levels at the site, it is considered that the site was unaffected.

5.1.5 The Environment Agency confirmed in correspondence that they do not hold any record of historical flooding in the vicinity of the site (see Appendix 5).

Fluvial Flooding

- 5.1.6 The Environment Agency Flood Map for Planning (see Figure 1) shows that the site is located wholly within Flood Zone 1, defined as an annual probability of fluvial flooding of less than 1 in 1,000 (<0.1%).
- 5.1.7 Areas to the south and west of the site, coincident with low-lying ground adjacent to the River Sherbourne, are shown to be located in Flood Zone 2 and Flood Zone 3, defined as an annual probability of between 1 in 1000 and 1 in 100 (0.1% 1.0%) and greater than 1 in 100 (>1.0%) respectively. Figure 2 focusses on the area in the vicinity of the site entrance of London Road (A4082). In all instances, areas of Flood Zone 2 and 3 are located below the existing ground levels of the site and are not considered to pose a risk of flooding to the site.
- 5.1.8 Drawing CA11485-038 'London Road Entrance Topography' shows that the existing entrance off London Road is approximately 5m higher than the River Sherbourne. Furthermore, vehicles must turn left upon leaving the site and progress eastwards, upslope away from the watercourse. Based on the significant level difference and route out of the site, it is considered there will be safe egress from the site during storm events up to and including the 1 in 1000 year extreme storm event (based on Flood Zone 1 classification in the vicinity of the site entrance).
- 5.1.9 The *'Climate Change Maps'* figure in the Coventry City SFRA applies a climate change factor to modelled watercourses to determine the extent of Flood Zone 3 including the impact of climate change. This figure shows that, with a climate change factor applied to the River Sherbourne model, Flood Zone 3 will not extend into the site area.
- 5.1.10 There are no other identified Ordinary Watercourses in the vicinity of the site which would affect this Flood Zone 1 classification and, based on the available information, the risk of fluvial flooding is considered to be **VERY LOW.**



5.1.11 Whilst the fluvial flood risk is considered to be Very Low, the risk to users of the site could be further minimised by registering for the phone, email and text message flood warnings service provided by the EA.

Surface Water Flooding (Pluvial Flooding)

- 5.1.12 The Environment Agency's 'Flood Risk from Surface Water' map shown in Figure 3 shows that the site is at a Very Low risk of surface water flooding (defined as a less than 1 in 1000 (0.1%) chance of flooding in any year). A small area within the existing industrial estate is shown to be at a Low risk (between 1 in 1000 and 1 in 100 annual probability) and a Medium risk (between 1 in 100 and 1 in 30 annual probability) of flooding. This area is adjacent to an existing wall within the industrial estate, where surface water runoff, flowing south-westwards and following the topography, could accumulate in a storm event. Ground levels in this area are lower than the main site area and any accumulated runoff in this location would not pose a flood risk to the main site.
- 5.1.13 Based on the available information, the risk of surface water flooding is considered to be **VERY LOW.**

Groundwater Flooding

- 5.1.14 Flooding can occur when prolonged rainfall causes the groundwater table to rise above ground level. Groundwater flooding can occur at the same time as flooding from other sources such as overland flow.
- 5.1.15 The 'Groundwater Flood Map' figure in the Coventry City Council SFRA shows that the 1km grid square in which the site is located has a 50% to 75% susceptibility to groundwater flooding. This susceptibility is defined as 'the proportion of each 1km grid square where geological and hydrogeological conditions show that groundwater might emerge' and that this 'does not show the likelihood of groundwater flooding occurring. This dataset covers a large area of land, and only isolated locations within the overall susceptible area are actually likely to suffer the consequences of groundwater flooding'.
- 5.1.16 The BGS Geology of Britain Viewer contains logs for a series of shallow boreholes between 1.0m and 1.2m in depth located within the Whitley Depot industrial estate. In all instances, groundwater was not encountered to the base of the borehole. These boreholes are located at elevations of approximately 80mAOD and within an area of similar geology. These are, therefore, considered to be representative of the ground conditions at the site.



Since no groundwater was encountered in any of the boreholes and that any groundwater is likely to be drawn down towards the nearby river, located over 5m lower than the site, the risk of groundwater flooding is considered to be **VERY LOW**.





Figure 3. Surface Water Flood Risk Map



Sewer Flooding

- 5.1.17 The public sewer records (see Appendix 1) show an egg-shaped brick public combined gravity sewer crossing south-eastwards through the site area. Manhole records show that the sewer is approximately 3m deep at the upstream manhole (ref SP34777701) and 7m deep at the downstream manhole, and at depths of up to 14m within the site area. As the sewer upstream of manhole ref: SP34777701 is understood to be abandoned it is assumed that there will be no incoming flows from off-site areas.
- 5.1.18 The majority of the main site area and surrounding land has been used as allotments for over 90 years. Due to this historical land use, and the relative distance of the allotments from other developments, it is considered that there are no private drainage networks serving this area.
- 5.1.19 Private drainage networks are, however, present within the industrial estate (Whitley Depot) and existing access road to the site off London Road, as shown on Drawing No 2/15816 *'Whitley Depot, London Road, Coventry'* and 2/14014 *'Whitley Depot London Road Main Yard Plan 2'*. Although the depths of the drains are not shown on the layout, this area is situated at a lower level than the developable site area and any flooding from this network would not have an impact on the proposed development.
- 5.1.20 Based on the available information, the risk of sewer flooding is considered to be **VERY LOW.**

Artificial Sources

- 5.1.21 Artificial sources of flooding include reservoirs, canals and any other impounded water body which is elevated above the level of the site. Flooding can occur when the impounding structures such as dams and embankments fail, when culverts become blocked, or during extreme rainfall events when the waterbodies overflow.
- 5.1.22 Environment Agency mapping shows that the site is not located within the maximum extents of a reservoir flooding event, and there are no other canals or impounded water bodies within the vicinity of the site. The risk of flooding from artificial sources is, therefore, discounted.

5.2 Flood Risk from the Proposed Development

5.2.1 New development can pose a risk of flooding to neighbouring properties and areas downstream of the site, often as a result of an increase in impermeable area which has the effect of increasing the rate and volume of surface water runoff. In addition,



climate change can be expected to cause an increase in rainfall intensity and surface water runoff over the lifetime of the development.

5.2.2 Flood risk can also be increased as a result of new development if the development reduces the floodplain storage area or alters flood flow paths, ultimately displacing flood water and resulting in an increased risk to the surrounding area.

Fluvial Flooding

5.2.3 As the proposed area to be redeveloped is located within Flood Zone 1, there will be no effect on the 1 in 100 year floodplain storage or flood flow paths and the risk of fluvial flooding will, therefore, not increase as a result of the proposed development.

Surface Water Runoff

- 5.2.4 New developments can also pose a risk of flooding to neighbouring properties and areas downstream of the site, often as a result of an increase in impermeable area which has the effect of increasing the rate and volume of surface water runoff, or by diverting existing overland flow routes crossing the site to off-site areas previously unaffected.
- 5.2.5 The proposed development can be divided into two areas: the 3.31ha main site area accommodating the proposed structures and hardstanding; and the 1.14ha narrow section of land, which includes the existing industrial estate access road. As this access road will not be redeveloped as part of the proposed development, there will be no increase in impermeable area and the impact on flood risk in this location is not considered further in this assessment.
- 5.2.6 Notwithstanding the previous usage of the main site area, and the presence of access tracks and demolished building foundations, it is considered that the site has the characteristics of a greenfield site with surface water runoff dispersing by a combination of overland flow, infiltration and evaporation. Surface water runoff will follow the natural topography, flowing generally westwards beyond the site boundary and ultimately discharging to the River Sherbourne or the adjacent compound area for the existing Energy from Waste site. It is considered that runoff within this compound area either discharges by overland flow to the River Sherbourne or via a private drainage network within this area.
- 5.2.7 The existing greenfield runoff rates for the 3.31ha main site area have been calculated following the IH124 methodology (see Appendix 6) and are summarised in Table 4 below.



| Table 4: Existing Runoff Rates | | | | |
|--------------------------------|--------------|--------------|---------------|--------------|
| Area Positively | | Greenfield | Runoff Rate | |
| Drained | 1 in 1 Year | 1 in 30 Year | 1 in 100 Year | QBAR |
| 3.31 ha | 12.3 l/s | 29.6 l/s | 38.0 l/s | 14.8 l/s |
| | 3.7 (l/s/ha) | 8.9 (l/s/ha) | 11.5 (l/s/ha) | 4.5 (l/s/ha) |

- 5.2.8 The proposed development will comprise the construction of a large building and a separate office building, plus associated hardstanding and access roads. The total impermeable area will be 2.77ha (consisting of 1.43ha of roof areas and 1.34ha of hardstanding/access roads). There are presently minimal areas of impermeable ground within the development, with existing paths consisting of bare ground rather than a formal concrete or asphalt construction. The proposed development will, therefore, increase the impermeable area at the site, potentially increasing the rate and volume of surface water runoff generated post-development.
- 5.2.9 As shown on Drawing No CA11485-057 '*Proposed Layout Option 8*', the existing sloped ground will be modified to create a relatively flat area ('plateau') on which the proposed buildings and hardstanding will be accommodated. The necessary use of retaining walls against the higher ground to the north and east has the potential to disrupt the south-westward progression of any surface water runoff, diverting these flows to other areas previously unaffected. In these areas it is considered that precautionary measures should be implemented to minimise any impact of the proposed development.

Climate Change

- 5.2.10 It is also necessary to take account of climate change for the lifetime of the development when assessing future flood risk. In assessing surface water runoff from the proposed development, the climate change predictions for peak rainfall intensity for the lifetime of the development need to be taken into account. Based on correspondence with Coventry City Council, the Lead Local Flood Authority, a 40% increase in peak rainfall intensity shall be used (see Appendix 4).
- 5.2.11 It is considered, therefore, that the risk of surface water flooding could increase as a result of the proposed development and that mitigation measures are required.

5.3 Flood Risk Mitigation Measures

Surface Water Management

5.3.1 To mitigate the increased risk of surface water flooding, it is proposed that surface water runoff from the development is discharged from site at a restricted rate, with



any flows in excess of this restricted rate being attenuated on site for all storm events up to and including the 1 in 100 year event with a 40% allowance for climate change.

- 5.3.2 Retaining walls (to retain higher ground) to the north and east of the development plateau have the potential to truncate existing westward overland flow routes. Drainage features will need to be constructed to ensure these flows are not diverted to areas previously unaffected.
- 5.3.3 The surface water management proposals for the site will ensure that flood risk due to surface water runoff is not increased as a result of the development.

5.4 Residual Risk

- 5.4.1 There is always a possibility of a storm event that exceeds the design standards of the proposed flood risk management measures for new developments. Potential risks include the exceedance of the surface water attenuation facilities during extreme storm events.
- 5.4.2 On-site attenuation will be provided for storm events up to and including the 1 in 100 year event with a 40% climate change allowance. For storm events with an annual probability of occurrence greater than this, the on-site attenuation may be exceeded.
- 5.4.3 The layout of the site will be designed so that exceedance flows are diverted away from the buildings and retained within the site as far as is possible, for example, with the use of kerbing or in areas adjacent to the retaining walls, with no risk to adjacent land. During storm events where rainfall exceeds the capacity even to do this, runoff will be directed off site to the watercourse in a controlled manner, away from vulnerable areas.



6 DRAINAGE STRATEGY

6.1 Surface Water Drainage Strategy

- 6.1.1 Surface water runoff from the development will be controlled on site to ensure that there is no increase in the risk of flooding to areas downstream of the site and to the development itself. To help achieve this, Sustainable Drainage Systems (SuDS) will be incorporated into the development to provide attenuation and water treatment. An Indicative Surface Water Management Plan for the site is included as Drawing No CA11485-064 'Indicative Drainage Strategy'.
- 6.1.2 The Building Regulations (2010) Part H stipulate a hierarchy for the disposal of surface water which should be followed as part of any surface water drainage design. This hierarchy is as follows:
 - i. an adequate soakaway or some other adequate infiltration system; or, where that is not practicable,
 - ii. a watercourse; or, where that is not practicable,
 - iii. a sewer.
- 6.1.3 In accordance with this hierarchy, it is proposed that SuDS features are designed to promote infiltration to the underlying bedrock. Detailed permeability testing will be carried out at the detailed design stage in accordance with BRE-365 in order to assess the permeability of the underlying ground and to determine the feasibility of infiltration drainage. If there is any risk of mobilising pollutants in existing Made Ground, SuDS features will be lined to prevent infiltration.
- 6.1.4 In accordance with the Building Regulations hierarchy, it is proposed to discharge the remaining surface water runoff, that exceeds the infiltration capacity of the ground, to the River Sherbourne via the existing private drainage network within the existing access road to the south-east of the main site area (see Drawing No 2/14014 'Whitley Depot London Road Main Yard Plan 2'). This is also in accordance with the requirements of Severn Trent Water (see Developer Enquiry response Appendix 2).
- 6.1.5 The rate of discharge will be restricted to 10.0 l/s for all storm events up to and including the 1 in 100 year storm event (plus 40% climate change allowance). This restricted rate is based on a post-development impermeable area of 2.77ha and a greenfield runoff rate of 3.6 l/s/ha. The greenfield runoff rate used gives a 20% betterment to the existing QBAR rate of 4.5 l/s/ha in accordance with the



requirements of the Coventry City Council 'Flood Risk Management and Drainage – Planning Standing Advice' (see Appendix 4).

6.1.6 As part of the proposed development, the ground in the vicinity of the buildings will be levelled to provide a relatively flat 'plateau' on which the main building, office building and the majority of the hardstanding will be located. Ground levels then fall southwards and eastwards to create a second 'plateau' adjacent to the weighbridge adjacent to the entrance to the MRF site, with the weighbridge situated approximately 1.5m lower the adjacent car park. It is proposed, therefore, to position the majority of the required surface water attenuation within these flatter open areas within the main plateau of the site as opposed to within the narrower section of land around the weighbridge immediately upstream of the final outfall at the site entrance. To achieve this, the discharge will be restricted both immediately downstream of the main plateau area and at the final outfall from the site.

Attenuation Estimates

- 6.1.7 Based on the proposed site layout (see Drawing No CA11485-057 '*Proposed Layout Option 8*'), there will be approximately 2.45ha of impermeable roof and hardstanding areas within the main plateau area, consisting of approximately 1.43ha of roof and 1.02ha of hardstanding. There will be approximately 0.32ha of impermeable access road at the second plateau around the weighbridge and site entrance.
- 6.1.8 Staged discharge will be used to best suit the site layout, with discharge from upper areas of the site restricted pro-rata based on the contributing area. This will allow the large areas of open ground within this area to be utilised for attenuation. The final outfall from the site will restrict the rate of discharge to the proposed 10 l/s with additional attenuation provided upstream.
- 6.1.9 Preliminary estimates of the volume of attenuation required have been made using the Storage Estimate module in the Causeway 'Flow' software package. The attenuation calculations are based on there being no infiltration to ground as a 'worst case' scenario. The infiltration rate for the site will, however, be confirmed at the detailed design stage by in situ soakaway testing in accordance with BRE Digest 365.
- 6.1.10 Attenuation calculations are contained in Appendix 7 and summarised in Table 6 below.



| Table 6: Preliminary Attenuation Estimates | | | | | | |
|--|--------------|----------------|--------------------|---------------------|--|--|
| | Contributing | | Attenuation Volume | | | |
| Location | Impermeable | Restricted | | 1 in 100 year + | | |
| | Δrea | Discharge Rate | 1 in 30 Year | 40% Climate | | |
| | Alcu | | | Change | | |
| Main Plateau | 2.45ha | 8.8 l/s | 967m ³ | 1,937m ³ | | |
| Weighbridge Area | 0.32ha | - | 125m ³ | 252m ³ | | |
| Total Attenuation 2.77ha 10.0 /ls- 1,092m ³ 2,189m ³ | | | | | | |
| Calculations are based on no infiltration to ground as a 'worst-case' scenario. | | | | | | |

Drainage from main plateau to discharge to final outfall at restricted 8.8l/s rate (pro-rata contributing area)

Sustainable Drainage System

- 6.1.11 Based on the minimal areas of open space within the site, is proposed that the majority of the required attenuation will be provided within underground geocellular units.
- 6.1.12 The geocellular storage will be unlined, where feasible, to allow infiltration to the underlying ground and will be sufficiently sized to provide temporary storage for all storm events up to and including the 1 in 100 year storm event, including a 40% allowance for climate change, where inflow rates exceed the infiltration rate.
- 6.1.13 The depth of made ground at the site will be confirmed as part of subsequent site investigation works and any infiltration SuDS features within the drainage network will be located at depths below the extent of the made ground to prevent the mobilisation of any contaminants within the material.
- 6.1.14 All surface water runoff will be conveyed through an appropriate number of SuDS features to provide the necessary water quality treatment. Generally, this will include two or three stages of treatment for runoff from roads and parking areas, and one stage of treatment for roof water runoff. The features will act as a 'SuDS Management Train' as described in the SuDS Manual.
- 6.1.15 To provide treatment to surface water runoff, all gullies and drainage channels will be fitted with silt traps and runoff from hardstanding will discharge via oil separators.
- 6.1.16 Lined filter trenches will be constructed around retaining walls in the north and east of the site which will convey intercepted flows south-westwards and northwestwards, as per the pre-development overland flow routes, around the development plateau to disperse within open ground.



6.2 Foul Water Drainage

6.2.1 It is proposed that foul water flows from the offices and processing building will discharge to the public foul sewer via manhole 8301 in the adjacent industrial estate. This is in accordance with the requirements of Severn Trent Water (see Developer Enquiry response, Appendix 2). An indicative route, subject to the location of incoming connections and flow rates from the buildings, is shown on Drawing No. CA11485-064 'Indicative Drainage Strategy'.



7 CONCLUSIONS

- 7.1.1 This report gives details of the Flood Risk Assessment, which has been carried out in accordance with the National Planning Policy Framework.
- 7.1.2 The proposed development comprises a Materials Recycling Facility and consists of a large processing building, office building and associated hardstanding. As part of the proposed development ground levels in northern and central areas of the site will be modified to produce a flat plateau.
- 7.1.3 The proposed development area is located within Flood Zone 1 according to the Environment Agency's current Flood Map for Planning and is defined as a 'Less Vulnerable' development in Table 2 of the NPPG. Less Vulnerable development is an appropriate development type within Flood Zone 1.
- 7.1.4 The risk of flooding from fluvial sources, surface water, groundwater and sewers is considered to be Very Low. The site is not considered to be at risk of flooding from artificial sources.
- 7.1.5 Flood risk management measures will be put in place to ensure that the risk of flooding to areas downstream of the site is not increased as a result of the development. This will include a surface water drainage strategy that will mimic greenfield performance for a range of storm events in accordance with the NPPF, Environment Agency requirements and local planning policy.
- 7.1.6 Surface water runoff will be managed in a sustainable manner and will be disposed of by infiltration to the ground, where possible, in accordance with the Building Regulations hierarchy. Where the infiltration rate is exceeded, surface water will be discharged to the River Sherbourne at a restricted rate of 10.0 l/s equivalent to 20% betterment of the existing greenfield (Qbar) runoff rate. On-site attenuation will be provided to accommodate flows up to and including the 1 in 100 year storm event, including a 40% climate change allowance.
- 7.1.7 As the risk of flooding to the site is considered to be Very Low, and the risk of flooding to surrounding areas will not increase as a result of the proposed development, it is considered that, from a flood risk and drainage perspective, the site is suitable for the type of development propose

APPENDICES

APPENDIX 1

Severn Trent Water Sewer Records



| Sewer Node | • | Sewer Pipe Data | | | | | | | | |
|-------------|-------------|--------------------|----------------------|------|------|-------|-------------|-------------|----------|--------------|
| REFERENCE | COVER LEVEL | INV LEVEL UPSTR | INV LEVEL DOWNSTR | PURP | MATL | SHAPE | MAX SIZE | MIN SIZE | GRADIENT | YEAR LAID |
| SP34775201 | 76.92 | 73.73 | 72.69 | s | nil | с | 525 | nil | 84.70 | nill |
| SP34775301 | 78.99 | 75.03 | 74.81 | s | nil | с | 525 | nil | 113.91 | nill |
| SP34775302 | 78.56 | 75.09 | 75.03 | S | nil | с | 300 | nil | 441.33 | nill |
| SP34775303 | 75.37 | 73.49 | 72.68 | с | nil | с | 225 | nil | 94.54 | nill |
| SP34775304 | 73.88 | 69.46 | 69.28 | С | VC | с | 225 | nil | 146.67 | nill |
| SP34775401 | 72.15 | 70.17 | 69.51 | с | VC | с | 225 | nil | 116.20 | nill |
| P34775402 | 72.19 | 70.35 | 70.17 | с | nil | с | 225 | nil | 226.67 | nill |
| P34775403 | 76.10 | 74.04 | 73.49 | с | nil | с | 225 | nil | 104.89 | nill |
| P34775501 | 74.61 | 73.67 | 71.75 | с | nil | с | 150 | nil | 16.22 | nill |
| P34775502 | 72.64 | 71.06 | 70.68 | с | nil | с | 225 | nil | 20.05 | nill |
| P34775503 | 73.11 | 70.56 | 70.35 | с | nil | с | 225 | nil | 233.76 | nill |
| P34775505 | nil | nil | nil | с | nil | с | 150 | nil | 0.00 | nill |
| \$P34775506 | nil | nil | 70.56 | с | nil | с | 150 | nil | 0.00 | nill |
| \$P34775507 | 73.45 | 72.42 | 71.24 | S | nil | с | 225 | nil | 15.58 | nill |
| SP34775508 | 72.63 | 71.24 | 69.29 | S | nil | с | 225 | nil | 10.57 | nill |
| P34775509 | 70.92 | 69.29 | nil | S | nil | с | 300 | nil | 0.00 | nill |
| SP34775510 | 71.71 | 70.44 | 70.56 | с | VC | с | 300 | nil | 0.00 | nill |
| P34775512 | 71.52 | 70.58 | 70.45 | с | VC | с | 300 | nil | 222.00 | nill |
| P34775513 | 71.54 | 70.66 | 70.58 | с | VC | с | 300 | nil | 190.38 | nill |
| P34775601 | 74.57 | 72.19 | 71.33 | с | VC | с | 150 | nil | 32.88 | nill |
| P34775602 | 73.87 | 71.33 | 71.06 | с | nil | с | 225 | nil | 281.22 | nill |
| SP34775604 | nil | nil | nil | s | nil | с | 150 | nil | 0.00 | nill |
| P34775605 | 75.33 | 74.40 | 72.03 | s | VC | с | 150 | nil | 16.80 | nill |
| P34775606 | 74.94 | 73.25 | 71.20 | S | nil | с | 225 | nil | 34.73 | nill |
| P34775607 | 74.24 | 72.31 | 72.20 | с | nil | с | 150 | nil | 38.55 | nill |
| P34775608 | 73.98 | 72.03 | 70.94 | S | nil | с | 150 | nil | 42.69 | nill |
| P34775609 | nil | nil | nil | с | nil | с | 150 | nil | 0.00 | nill |
| P34775610 | nil | nil | nil | S | nil | с | 150 | nil | 0.00 | nill |
| SP34775611 | nil | nil | nil | S | nil | с | nil | nil | 0.00 | nill |
| SP34775613 | nil | nil | 71.75 | с | nil | с | nil | nil | 0.00 | nill |
| P34775615 | nil | nil | nil | с | nil | с | nil | nil | 0.00 | nill |
| SP34775616 | nil | nil | nil | S | nil | с | 100 | nil | 0.00 | nill |
| SP34775704 | 75.69 | 72.31 | 69.87 | S | со | с | 375 | nil | 43.94 | nill |
| SP34775705 | 75.93 | 72.87 | 70.63 | с | nil | с | 225 | nil | 34.04 | nill |
| SP34776101 | 75.01 | 72.61 | 71.98 | S | nil | с | 600 | nil | 156.95 | nill |
| SP34776301 | 73.63 | 70.16 | 69.91 | с | VC | с | 225 | nil | 32.00 | nill |
| SP34776302 | 73.27 | 69.20 | 68.45 | с | VC | с | 225 | nil | 9.61 | nill |
| P34776303 | 73.24 | 65.54 | 65.29 | с | CSB | с | 3050 | nil | 1118.76 | nill |
| P34776304 | 74.65 | 69.28 | nil | с | VC | с | 225 | nil | 0.00 | nill |
| P34776305 | nil | nil | nil | с | VC | С | 225 | nil | 0.00 | nill |
| P34776306 | 74.48 | 70.54 | 70.19 | с | VC | с | 225 | nil | 40.91 | nill |
| P34776501 | nil | nil | 67.39 | s | nil | С | 300 | nil | 0.00 | nill |
| P34776503 | nil | nil | nil | s | nil | с | 300 | nil | 0.00 | nill |
| SP34776601 | 73.93 | 70.64 | 67.85 | с | nil | С | 225 | nil | 10.44 | nill |
| SP34776602 | 70.78 | 69.18 | nil | s | nil | с | 225 | nil | 0.00 | nill |
| SP34776603 | 72.50 | 70.94 | 69.18 | S | nil | с | 225 | nil | 39.05 | nill |

MATERIALS

| - | - NONE | PE | - POLYETHLENE |
|-----|--------------------------------|-----|-----------------------|
| AC | - ASBESTOS CEMENT | PF | - PITCH |
| BR | - BRICK | PP | - POLYPROPYLENE |
| СС | - CONCRETE BOX CULVERT | PSC | - PLASTIC STEEL COMPO |
| CI | - CAST IRON | PVC | - POLYVINYL CHLORIDE |
| со | - CONCRETE | RPM | - REINFORCED PLASTIC |
| CSB | - CONCRETE SEGMENTS (BOLTED) | SI | - SPUN (GREY) IRON |
| CSU | - CONCRETE SEGMENTS (UNBOLTED) | ST | - STEEL |
| DI | - DUCTILE IRON | U | - UNKNOWN |
| GRC | - GLASS REINFORCED CONCRETE | VC | - VITRIFIED CLAY |
| RP | - GLASS REINFORCED PLASTIC | ххх | - OTHER |
| MAC | - MASONRY IN REGULAR COURSES | | |

MAR - MASONRY RANDOMLY COURSED

SHAPE PURPOSE
 POLYETHLENE
 C
 - CIRCULAR
 C
 - COMBINED

 - PITCH
 E
 - EGG SHAPED
 E
 - FINAL EFFLUENT
 - POLYPROPYLENE O - OTHER
 - PLASTIC STEEL COMPOSITE
 R
 - RECTANGLE
 L
 - SLUDGE

 - POLYVINYL CHLORIDE
 S
 - SQUARE
 S
 - SURFACE WATER
 - REINFORCED PLASTIC MATRIX T - TRAPEZOIDAL - SPUN (GREY) IRON U - UNKNOWN

TABULAR KEY

A. Sewer pipe data refers to downstream sewer pipe.

F - FOUL

- B. Where the node bifurcates (splits) X and Y indicates downstream sewer pipe.
- C. Gradient is stated a 1 in...

| e | BEVERN TRENT WATER | Severn Trent Water Limited Asset Data Management PO Box 5344 Coventry CV3 9FT Telephone: 0845 601 6616 | | |
|----------------------|--------------------------|---|--|--|
| SE | WER RECORD | DATA TABLE | | |
| O/S Map scale: | 1:3500 | This map is centred upon: O / S Grid reference: | | |
| Date of issue: | 09.01.19 | x: 434845 | | |
| Sheet No. | 2 of 3 | y: 277541 | | |
| Discloimor Statement | | | | |

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| 0555051/05 | | INV LEVEL | INV LEVEL | | | | MAX | MIN | | YEAR |
|------------|-------------|-----------|-----------|----------|------|--------|------|---------|----------|----------|
| REFERENCE | COVER LEVEL | UPSTR | DOWNSTR | PURP | MATL | SHAPE | SIZE | SIZE | GRADIENT | LAID |
| SP34776604 | 72.73 | 71.20 | 70.94 | S | nil | C | 225 | nil | 38.65 | nill |
| SP34776605 | nil | nil | 71.64 | С | nil | C | 225 | nil | 0.00 | nill |
| P34776606 | 71.18 | 65.77 | nil | С | CSB | С | 3050 | nil | 0.00 | nill |
| P34776702 | 76.03 | 70.61 | nil | С | VC | С | 225 | nil | 0.00 | nill |
| P34776703 | 71.31 | 69.87 | 69.17 | S | со | С | 450 | nil | 39.01 | nill |
| P34776801 | 70.41 | 66.56 | nil | С | nil | С | nil | nil | 0.00 | nill |
| P34776801 | 70.41 | 66.51 | nil | С | nil | С | nil | nil | 0.00 | nill |
| P34776802 | 70.53 | 65.98 | 65.77 | С | CSB | С | 3050 | nil | 1017.14 | nill |
| P34777102 | 72.13 | nil | nil | nil | nil | nil | nil | nil | 0.00 | nill |
| P34777103 | nil | nil | nil | nil | nil | nil | nil | nil | 0.00 | nill |
| P34777701 | 72.35 | 69.95 | 69.99 | С | nil | с | nil | nil | 0.00 | nill |
| P34777706 | nil | nil | nil | s | nil | с | 600 | nil | 0.00 | nill |
| P34777801 | 71.14 | 69.57 | 68.64 | S | nil | С | 900 | nil | 36.39 | nill |
| P34777803 | 72.30 | 70.30 | 69.37 | С | nil | с | 750 | nil | 119.40 | nill |
| P34778201 | 71.37 | 67.70 | 67.60 | с | со | с | 1125 | nil | 276.60 | nill |
| P34778203 | 70.79 | 67.86 | 67.70 | С | nil | С | 900 | nil | 92.94 | nill |
| P34778204 | nil | nil | 67.86 | с | CI | с | 900 | nil | 0.00 | nill |
| P34778205 | 72.26 | 69.35 | nil | с | CI | с | 900 | nil | 0.00 | nill |
| P34778301 | 76.84 | 69.57 | 69.60 | с | со | E | 1520 | 1020 | 0.00 | nill |
| P34778501 | 84.83 | 69.99 | 69.56 | с | со | E | 1520 | 1020 | 534.56 | nill |
| P34778701 | 71.17 | nil | 69.57 | s | nil | с | 900 | nil | 0.00 | nill |
| P34778701 | 71.17 | nil | nil | S | nil | С | 600 | nil | 0.00 | nill |
| P34778801 | 72.17 | 70.03 | nil | s | nil | С | 1050 | nil | 0.00 | nill |
| P34778802 | 72.31 | 70.75 | 70.35 | s | nil | c | 600 | nil | 25.00 | nill |
| P34778803 | nil | nil | 70.17 | s | nil | c | 900 | nil | 0.00 | nill |
| P34779201 | 74 17 | 69 59 | 69.60 | C. | BR | F | 1635 | 1020 | 0.00 | nill |
| P34779202 | 74.05 | 69.50 | 69.35 | C. | 0 | | 900 | nil | 211.87 | nill |
| P34779801 | 72.88 | 70 54 | 70.47 | s | nil | C. | 900 | nil | 542 14 | nill |
| P34779802 | 72.32 | 70.59 | 70.57 | 6 | nil | | 900 | nil | 1011.00 | nill |
| P3/779803 | 73.32 | 70.83 | nil | 6 | nil | C C | 900 | nil | 0.00 | nill |
| P34779804 | 72.59 | 71.12 | 70.85 | <u>s</u> | 0 | с С | 600 | nil | 170.26 | nill |
| P3/770805 | 72.64 | 71.23 | 71.13 | 4 | nil | | 600 | nil | 244.10 | nill |
| D34770806 | 73.03 | 71.25 | 71.07 | 6 | | | nil | nil | 244.10 | rill |
| - 3+119000 | 72.02 | 71.50 | 70.97 | | nil | | 450 | nii | 222.22 | |
| F34//9800 | 73.03 | 72.42 | 10.87 | <u> </u> | nii | | 450 | nii | 30.37 | |
| -34//98U/ | 73.95 | 72.43 | | <u> </u> | | | 300 | nii | 0.00 | nill |
| -34779808 | 73.99 | /2.4/ | /1.2/ | 5 | VC | | nii | nii | 55.70 | nill |
| -34779809 | 73.34 | 70.44 | nil | S | nil | C | 900 | nil | 0.00 | nill |
| P35770701 | nil | nil | nil | nil | nil | nil | nil | nil | 0.00 | nill |
| P35770702 | nil | nil | nil | nil | nil | nil | nil | nil | 0.00 | nill |

| MATERIALS | | | | SHAPE | | PURPOSE | | SI | EVERN | Sev | ern Trent Water Limited et Data Management |
|-----------|--------------------------------|--|-----------------------------|-------|--|---------|--|--|--|--|--|
| - | - NONE | PE | - POLYETHLENE | С | - CIRCULAR | С | - COMBINED | Т | RENT | Cov CV | BOX 5344 /entry 3.9FT |
| AC | - ASBESTOS CEMENT | PF | - PITCH | Е | - EGG SHAPED | Е | - FINAL EFFLUENT | V | /ATER | Tele | ephone: 0845 601 6616 |
| BR | - BRICK | PP | - POLYPROPYLENE | 0 | - OTHER | F | - FOUL | SEV | VER RECORD D | DATA 7 | ABLE |
| CC | - CONCRETE BOX CULVERT | PSC | - PLASTIC STEEL COMPOSITE | R | - RECTANGLE | L | - SLUDGE | | 1.0500 | | |
| CI | - CAST IRON | PVC | - POLYVINYL CHLORIDE | s | - SQUARE | s | - SURFACE WATER | O/S Map scale: | 1:3500 | Thi 0 / | is map is centred upon: S Grid reference: |
| со | - CONCRETE | RPM | - REINFORCED PLASTIC MATRIX | т | - TRAPEZOIDAL | | | Date of issue: | 09.01.19 | x: | 434845 |
| CSB | - CONCRETE SEGMENTS (BOLTED) | SI | - SPUN (GREY) IRON | U | - UNKNOWN | | | | | | |
| CSU | - CONCRETE SEGMENTS (UNBOLTED) | ST | - STEEL | | | | | Sheet No. | 3 of 3 | у: | 277541 |
| DI | - DUCTILE IRON | U | - UNKNOWN | | TABULAR KEY | | | Disclaimer Statement: | | | |
| GRC | - GLASS REINFORCED CONCRETE | VC | - VITRIFIED CLAY | Α. | Sewer pipe data refers to downstream sewer pipe. | | 1. Do not scale off this Map. 2. This map and any information supplied with it is furnished as a general guide, is only valid at the date of issue and no warranty as to it correctness is given or implied. In particular this Map and any information abown on it must not be relied upon in the event of any correctness is given or implied. In particular this Map and any information abown on its must not be relied upon in the event of any correctness is given or implied. In particular this Map and any information abown on its must not be relied upon in the event of any correct the severe part of attribution systems. | | | | |
| RP | - GLASS REINFORCED PLASTIC | XXX | - OTHER | в. | Where the node bifurcates (splits) X and Y indicates downstream sewer pipe. | | 3. On 1 October 2011 most private severes and private lateral drams in Severn Trent Water's severage area, which were connected to a public sever and at 1 July 2011. Inselfered to the ownership of Severn Trent Water and Secura public lateral drams. A further transfer takes place on 1 October 2012 (date to be confirmed). Phylee pompaging 2016s, which from part of these severes or lateral drams, will transfer to the ownership of Severn Trent Water on Secura 1 July 2011. The private private severes on the set of atoms, will transfer to the ownership of Severn Trent Water on control of the severes or lateral drams. | | | | |
| MAC | - MASONRY IN REGULAR COURSES | ASONRY IN REGULAR COURSES C. Gradient is stated a 1 in | | | Severn Trent Water does not possess complete records of these assets. These assets may not be displayed on this Map. 4. Reproduction by permission of Ordnance Survey on behalf of HMSO. © Crown Copyright and database right 2004. All rights res | | | | | | |
| MAR | - MASONRY RANDOMLY COURSED | | | | | | | Ordnance Survey licence number 10 document is provided for reference p should be made from it. | 0018202. Document users other than suppose only and is subject to copyright, the | ievern Trent Wal terefore, no furth | ter business users are advised that this her copies should be made from it. |

APPENDIX 2

Severn Trent Water Developer Enquiry Response

WONDERFUL ON TAP



Wardell Armstrong Sir Henry Doulton House Forge Lane Etruria Stoke on Trent ST1 5BD

FAO: Bryn Griffiths

Severn Trent Water Ltd Leicester Water Centre Gorse Hill Anstey Leicester LE7 7GU

Tel: 024 777 16843

www.stwater.co.uk net.dev.east@severntrent.co.uk

Contact: Asset Protection East (waste water)

Your Ref: Our Ref: 8343120

28th January 2019

Dear Sir,

Land off Shortley Road, Coventry, West Midlands, CV3 4AE Proposed Recycling Facility (434812, 277541)

I refer to your 'Development Enquiry Request' in respect of the above named site. Please find enclosed the sewer records that are included in the fee together with the Supplementary Guidance Notes (SGN) which refer to surface water disposal from development sites.

Protective Strips

Due to recent change in legislation, there could be sewers, which have transferred over to the Company that are not shown on the statutory sewer records, but are located on your clients land. These sewers will have protective strips that we will not allow to be built over. The sewers could be identified whilst the land is being surveyed. If this is the case, please contact us for further guidance upon discovery.

Please note there is a 1520mm x 1020mm dia public combined sewer crossing diagonally through the site north to south. I note from the submitted drawing that the intention is to build over this sewer. Severn Trent do not allow any new buildings over public sewers, and certainly not one that is this large.

However, I note from the sewer records that the sewer feeding into this large combined sewer to the north of the site (at manhole 7701) is shown on the record as an abandoned sewer. This incoming sewer looks to have been diverted in another direction at manhole 7803 (please see attached plan – abandoned sewer north). I will need to

WONDERFUL ON TAP



investigate this further to understand why the remainder of this sewer crossing through the site has not been abandoned, it may still be in use or is being mothballed in case of further future use. This may take several days to find out that information.

If the above sewer is abandoned, then it will need to be officially divested as a public sewer and the ownership handed over to the developer / land owner who will then be able to build over the sewer as it will be then private, subject to the proposed build and the sewer position under that being made acceptable to building regulations. Sewer grouting may be required to achieve this, if it has not already been carried.

I will write to you again on this matter within the next working week.

Foul Water Drainage

Subject to the previous section, if the sewer is abandoned, a suitable connection point could be at manhole 8301 in the neighbouring industrial property (Whitley Depot) and with their consent for a connection, or alternatively a new sewer constructed in the proposed new access road to the south and into the sewer where it will be crossing that access road, between MHs 8205 and 8204. The sewer at this point is 900mm dia. The type of connection at this point will be determined by the Developer Services S106 New Connections team.

Surface Water Drainage

Under the terms of Section H of the Building Regulations 2010, the disposal of surface water by means of soakaways should be considered as the primary method. If this is not practical and no watercourse is available as an alternative, the use of sewerage should be considered. In addition, other sustainable drainage methods should also be explored before a discharge to the public sewerage system is considered.

If ground conditions are not favourable, for soakaways and other SUDs techniques, evidence should be submitted. The evidence should be either percolation test results or by the submission of a statement from the SI consultant (extract or a supplementary letter). This would satisfy the SGN (enclosed).

Subject to the above, I believe that there is unlikely to be any formal drainage of the existing allotment site to the public sewer with all buildings being temporary types such as sheds, greenhouses etc. It is therefore very likely any surface water run-off from footpaths and sheds will drain to ground and eventually will run-off to the adjacent watercourse.

WONDERFUL ON TAP



For this application of a new build all surface water run-off from the new buildings and surfaces shall be discharged to the nearby watercourse. This will need to be determined for quantity and quality by the Environment Agency. Please note some form of onsite treatment or filtering may be required depending upon the nature of the materials to be processed within this site and the nature of the sw runoff from the road/storage areas.

Where the EA raise concerns for quality reasons to discharge to the watercourse a Trade Effluent (TE) licence can be applied for from Severn Trent, details for which are on our website at <u>www.stwater.co.uk</u>. There will be a fee for the TE licence and ongoing charges levied against the business owners. Please note wherever possible any roof rainwater shall be disposed of to the watercourse and not the public sewer.

Any flows generated by the site in excess of the permitted discharge rate will have to be attenuated within the development site, subject to EA / STW TE requirements.

Connections

For any new connections including the use, reuse and indirect to the public sewerage system, the developer will need to submit Section 106 application. Our Developer Services department are responsible for handling all such enquiries and applications. To contact them for an application form and associated guidance notes please call 0800 707 6600 or download from www.stwater.co.uk

Please quote the above reference number in any future correspondence (including e-mails) with STW Limited. Please send **all correspondence** to the <u>net.dev.east@severntrent.co.uk</u> email inbox address, a response will be made within 15 days.

Please note that Developer Enquiry responses are only valid for 6 months from the date of this letter.

Yours sincerely,

bolic

Keith Baker Senior Evaluation Technician Asset Protection East (wastewater) Asset Data Wholesale Operations
WONDERFUL ON TAP



SUPPLEMENTARY GUIDANCE NOTES RELATING TO DISPOSAL OF SURFACE WATER



Introduction

The purpose of this guidance note is to provide advice to applicants when completing the surface water drainage design for a new development, both for Greenfield and Brownfield sites. This does not affect foul drainage disposal which should be discussed with Severn Trent as early as possible to ensure additional flows can be accommodated without undue delay to the development.

Lead Local Flood Authority (LLFA) Consultation

Since April 2015, the LLFA have assumed the role of being a statutory consultee in the planning process for developments of 10 dwellings or more; or equivalent non-residential and/or mixed development. The LLFAs role is vital to ensure that surface water disposal on new development is adequately assessed so that the local planning authority can satisfy themselves that drainage proposals are satisfactory and to make sure, through the use of planning conditions or planning obligations, that there are clear arrangements in place for future maintenance of sustainable drainage systems (SuDS) over the lifetime of the development. This will also ensure surface water disposal aligns with local planning policies, flood risk strategies and national policies, such as the National Planning Policy Framework (NPPF).

It is strongly recommend that the LLFA are involved in early pre-application discussions when the development of a site is initially being considered. Pre-application discussions will help to ensure that SuDS are appropriately considered ahead of or as part of preliminary development layouts, and that they are fully integrated into the final development layout. Whilst Severn Trent are willing to advise on sewerage availability this does to negate the planning requirement relating to adequacy of SuDS on new development.

SuDS Hierarchy

Severn Trent is fully supportive of the fundamental SuDS principle that priority should be given to managing surface water as close to source as possible. In accordance with national standards and guidance a sequential series of checks should be undertaken to ensure the relevant SuDS features are being proposed whereby (in order of priority) rainwater re-use, infiltration to ground and controlled discharge to a water body are properly considered ahead of any <u>controlled</u> connection to a culverted watercourse/other drainage system or public surface water sewer.

A controlled connection to a public combined/foul sewer would only be considered under rare exceptional circumstances where all other options have been completely exhausted. Acceptance of surface water into a combined sewer is not only unsustainable because of the need to convey/treat rainwater but is also takes away existing capacity which could constraint the connection of foul flows on future development. It is also possible that connection of additional surface water flows will require capacity upgrades to the existing sewerage system which may delay development.

Connection to a Public Sewer

Whilst Severn Trent will be able to provide advice on potential public surface water sewer connection options, it is essential that a developer contacts the LLFA as early as possible to discuss surface water disposal as they will be able to provide guidance on surface water flood risk policy which may influence SuDS requirements. It is strongly recommended that LLFA discussions take place <u>before</u> contacting Severn Trent. Where the outcome of LLFA discussions concludes that a controlled discharge to the public sewerage system is the only viable option then Severn Trent would be pleased to discuss sewer connection options, satisfied that the LLFA have been consulted in line with their surface water management role and in their capacity as statutory consultee.

Evidence must be provided to demonstrate why the sequential SuDS checks have concluded that a connection to the public sewer is required. This must include a Site Investigation Report including percolation test data/graphs/calculations/results together with relevant correspondence with the LLFA.

Design Standards

Surface water disposal design should consider the interactions between the adoptable sewer design criteria based on a 30 year design storm (outlined in 'Sewers For Adoption') and the "Non-statutory technical standards for SuDS" requirement to restrict discharge from a site up to and including the 1 in 100 year critical storm event plus an allowance for climate change as required by the LLFA.

For Greenfield development, the peak runoff rate should never exceed the peak pre-development run-off rates/volumes for the same rainfall event irrespective of the design storm duration consistent with the national non-statutory technical standards. For developments which were previously developed (Brownfield), the peak runoff rate must be as close as reasonably practicable to the greenfield runoff rate from the development for the same rainfall event, but should never exceed the rate of discharge from the development prior to redevelopment again for the same rainfall event. This requirement to remove pre-development surface water discharges to the sewerage system will help remove capacity constraints and aid future development.

To establish the pre-development run-off rates a detailed existing drainage survey will be required indicating pipe locations including sizes and levels, impermeable area connectivity to each pipe and topographical information to support existing drainage assumptions. Photographs of the existing buildings and surface features should be provided and where necessary a CCTV sewer survey should be provided to support the drainage survey to demonstrate connectivity.

In line with 'Sewers for Adoption', the drainage system must be designed so that, unless an area is designated to hold and/or convey water as part of the design, flooding does not occur on any part of the site for a 1 in 30 year rainfall event. For higher storm return periods the drainage system must be designed so that, unless an area is designated to hold and/or convey water as part of the design, flooding does not occur during a 1 in 100 year rainfall event in any part of: a building (including a basement); or in any utility plant susceptible to water (e.g. pumping station, electricity substation, water booster station) within the development.

Small Developments

Whilst developments of fewer than 10 dwellings (or their equivalent) are excluded from the post April 2015 planning requirements the underlying principles regarding sustainable surface water management are still valid. The collective impacts of surface water discharges from smaller developments can have an adverse impact on flood risk, especially in smaller rural catchments where smaller sewerage systems are more susceptible to increases in surface water inflow. On small developments infiltration to ground and peak flow attenuation must be considered to mitigate flood risk in the community but where a sewer connection is envisaged then the developer is recommended to discuss surface water disposal options with Severn Trent as early as possible.

Contact

For further assistance please contact our Asset Protection teams via: <u>net.dev.west@severntrent.co.uk</u> (Birmingham & Black Country, Staffordshire, Shropshire, Worcestershire, Gloucestershire, Herefordshire, Powys) <u>net.dev.east@severntrent.co.uk</u> (Derbyshire, Leicestershire, Nottinghamshire, Warwickshire, Coventry)

Additional Guidance Notes

If you experience difficulty in the provision of off-site sewers to serve your proposed development, an application for requisition sewers under Section 98 Water Act 2003 may be appropriate on request to this office.

If there are existing public sewers within the curtilage of the development site that may affect the proposed development, the option to divert them under Section 185 Water Act 2003 may be available. All costs incurred would lay with the Applicant.

All potentially adoptable sewers must be designed and constructed in accordance with the guidelines in Sewers for Adoption (6th Edition), after 1st May 2006. A Severn Trent Water Addendum for Foul Sewage Pumping Stations will be available at <u>www.wrcplc.co.uk/sfa</u>.

If the sewers are to be offered for adoption or if the development works could affect the public sewerage system, the Developer should approach Severn Trent Water Ltd to discuss their proposals in detail. This is to ensure the Developer is aware of the Company's requirements which could affect the development design and/or programme.

In cases where the complexity of both the existing receiving sewerage system and the proposed additional sewerage necessitates the construction of a suitable computer model, Severn Trent Water will carry this out where required.

Severn Trent Water has no knowledge of any specific land drainage issues involving this site. The Developer is to contact and seek approval of The Environment Agency, Local Authority etc. regarding any means of surface water disposal to the land drainage system, required attenuation, discharge consent etc.

All enquiries with respect to the supply of sewer records only should be directed to Severn Trent Water Limited, Asset Data Management, PO Box 5344, Coventry. CV3 9FT (Tel. 0845 601 6616).

Asset Protection Waste Water East



| Sewer Node | | Sewer Pipe Data | | | | | | | | |
|------------|-------------|--------------------|----------------------|------|------|-------|-------------|-------------|----------|--------------|
| REFERENCE | COVER LEVEL | INV LEVEL UPSTR | INV LEVEL DOWNSTR | PURP | MATL | SHAPE | MAX SIZE | MIN SIZE | GRADIENT | YEAR LAID |
| SP34776703 | 71.31 | 69.87 | 69.17 | s | со | с | 450 | nil | 39.01 | nill |
| SP34776902 | 72.24 | 67.99 | 66.51 | с | со | С | 900 | nil | 52.34 | nill |
| SP34776902 | 72.24 | 67.91 | 66.48 | с | со | с | 900 | nil | 54.17 | nill |
| SP34777701 | 72.35 | 69.95 | 69.99 | с | nil | с | nil | nil | 0.00 | nill |
| SP34777706 | nil | nil | nil | s | nil | с | 600 | nil | 0.00 | nill |
| SP34777801 | 71.14 | 69.57 | 68.64 | s | nil | с | 900 | nil | 36.39 | nill |
| SP34777803 | 72.30 | 70.30 | 69.37 | с | nil | С | 750 | nil | 119.40 | nill |
| SP34778701 | 71.17 | nil | 69.57 | s | nil | С | 900 | nil | 0.00 | nill |
| SP34778701 | 71.17 | nil | nil | s | nil | С | 600 | nil | 0.00 | nill |
| SP34778801 | 72.17 | 70.03 | nil | s | nil | С | 1050 | nil | 0.00 | nill |
| SP34778802 | 72.31 | 70.75 | 70.35 | s | nil | С | 600 | nil | 25.00 | nill |
| SP34778803 | nil | nil | 70.17 | s | nil | С | 900 | nil | 0.00 | nill |
| SP34778900 | nil | nil | nil | F | VC | С | nil | nil | 0.00 | nill |
| SP34779801 | 72.88 | 70.54 | 70.47 | s | nil | С | 900 | nil | 542.14 | nill |
| SP34779802 | 72.32 | 70.59 | 70.57 | s | nil | С | 900 | nil | 1011.00 | nill |
| SP34779803 | 73.32 | 70.83 | nil | s | nil | С | 900 | nil | 0.00 | nill |
| SP34779804 | 72.59 | 71.12 | 70.85 | s | со | С | 600 | nil | 170.26 | nill |
| SP34779805 | 72.64 | 71.23 | 71.13 | s | nil | С | 600 | nil | 244.10 | nill |
| SP34779806 | 73.03 | 71.36 | 71.27 | s | со | С | nil | nil | 222.22 | nill |
| SP34779806 | 73.03 | 71.57 | 70.87 | s | nil | С | 450 | nil | 36.37 | nill |
| SP34779807 | 73.95 | 72.43 | nil | s | nil | с | 300 | nil | 0.00 | nill |
| SP34779808 | 73.99 | 72.47 | 71.27 | s | VC | с | nil | nil | 55.70 | nill |
| SP34779809 | 73.34 | 70.44 | nil | s | nil | с | 900 | nil | 0.00 | nill |
| SP34779901 | nil | nil | nil | s | U | U | 300 | nil | 0.00 | nill |
| SP34779902 | nil | nil | nil | s | U | U | 300 | nil | 0.00 | nill |
| SP34779903 | nil | nil | nil | s | U | U | 225 | nil | 0.00 | nill |

| ← X X X Abandoned Gravity Sewer |
|--|
| Private Combined Gravity Sewer |
| Private Foul Gravity Sewer |
| Private Surface Water Gravity Sewer |
| Public Combined Gravity Sewer |
| ► ► Public Foul Gravity Sewer |
| Public Surface Water Gravity Sewer |
| Trunk Combined Gravity Sewer |
| ►··►· Trunk Foul Use Gravity Sewer |
| Trunk Surface Water Gravity Sewer |
| Combined Use Pressurised Sewer |
| Foul Use Pressurised Sewer |
| └── → ── → Surface Water Pressurised Sewer |
| 🛏 — — Highway Drain |
| Combined Lateral Drain (SS) |
| Foul Lateral Drain (SS) |
| ► - ► Surface Water Lateral Drain (SS) |
| |
| All Private Sewers are shown in magenta All section 104 sewers are shown in green |
| All Sewers that have been transferred to Severn Trent |
| Water after the 1 st October 2011, but have not been surveyed an |
| commed by Severn ment water are shown in orange |

| Dilled Use Flessulised Sewel | and the second s | |
|-------------------------------|--|--|
| Use Pressurised Sewer | DS | Disposal Site |
| ace Water Pressurised Sewer | STW | Sewage Treatment Works |
| way Drain | | Lisua Other |
| bined Lateral Drain (SS) | | Housing, Other |
| Lateral Drain (SS) | | Pipe Support Structure |
| ace Water Lateral Drain (SS) | | |
| | | Sewage Pumping Facility |
| shown in green | \boxtimes | Sewer Facility Connection Inlet / Outlet |
| I transferred to Severn Trent | <u>c 3</u> | , - |
| Water are shown in orange | | |

| | Culverted Watercourse | • |
|---|--------------------------------------|------------|
| 0 0 0 | - Cable, Earthing | - 62 |
| > | Cable Junction | |
| | Cable, Optical Fibre/Instrumentation | \bigcirc |
| | Cable, Low Voltage | • |
| | Cable, High Voltage | • |
| +++++++++++++++++++++++++++++++++++++++ | Cable, Other | |
| B | Housing, Building | Mc- |
| K | Housing, Kiosk | - |
| DS | Disposal Site | п |
| STW | Sewage Treatment Works | - |
| | Housing, Other | Π |
| - | Pipe Support Structure | |

| | Blind Shaft |
|---|----------------------|
| • | Combined Use Manhole |

| | Flushing Chamber |
|---|------------------|
| • | Foul Use Manhole |
| • | Grease Trap |
| * | Head Node |
| - | Hydrobrake |
| | Lamphole |
| - | Outfall |
| | Overflow |

| = | Penstock |
|---|----------|

Petrol Interceptor

| Xa. | 4 | | | |
|------------|--|---|-------------------------|---|
| - | Sewer Chemical Injection Point | MATERIALS | CATEGORIES | |
| ÷. | Sewer Junction | NONE AC - ASBESTOS CEMENT | | |
| ÷. | Sewerage Air Valve | BR - BRICK CC - CONCRETE BOX CULVERT | | |
|) . | Sewerage Hatch Box Point | CO - CONCRETE CSB - CONCRETE SEGMENTS (BOLTED) | | |
| - | Sewerage Isolation Valve | CSU - CONCRETE SEGMENTS (UNBOLTED) DI - DUCTILE IRON | | |
| ~ | Soakaway | GRC - GLASS REINFORCED CONCRETE GRP - GLASS REINFORCED PLASTIC MAC - MASONRY IN REGULAR COURSES | N A | O/S Map scale: |
| 0 | Surface Water Manhole | MAR - MASONRY RANDOMLY COURSED PE - POLYETHLENE | W E | Data of issues |
| | Vent Column | PF - POLYPROPYLENE PSC - PLASTIC STEEL COMPOSITE | \checkmark | Date of issue: |
| | Waste Water Storage | PVC - POLYVINYL CHLORIDE RPM - REINFORCED PLASTIC MATRIX | S | Sheet No. |
| ++++ | Pre-1937 Properties | SI - SPUN (GREY) IRON ST - STEEL U - UNKNOWN | W - WEIR C - CASCADE | |
| | | VC - VITRIFIED CLAY | DB - DAMBOARD | Disclaimer Statement: |
| | TABULAR KEY | XXX - OTHER | FV - FLAP VALVE | 2. This map and any information s |
| • | Sewer pipe data refers to downstream | SHAPE PURPOSE | BD - BACK DROP | and any information shown on it m the purposes of determining the s |
| ~ | sewer pipe. | C - CIRCULAR C - COMBINED | HD - HIGHWAY DRAIN | 3. On 1 October 2011 most pri |
| в. | Where the node bifurcates (splits) X and Y | E - EGG SHAPED E - FINAL EFFLUENT | S104 - SECTION 104 | ownership of Severn Trent Water Private pumping stations, which for |
| - | indicates downstream sewer pipe. | R - RECTANGLE L - SLUDGE | | Severn Trent Water does not poss |
| С. | Gradient is stated a 1 in | S - SQUARE S - SURFACE WATER T - TRAPEZOIDAL U - UNKNOWN | | Reproduction by permission of Document users other than Sever should be made from it. |

| SEVERN TRENT WATER | | Severn Trent Water Limited Asset Data Management PO Box 5344 Coventry CV3 9FT Telephone: 0845 601 6616 | | | |
|--------------------------|---------|---|--------|--|--|
| SEV | VER REC | ORD (Tabu | lar) | | |
| 1:1750 | | This map is centred upon: O / S Grid reference: | | | |
| 1 of 1 | 1 of 1 | x : | 434842 | | |
| | | у: | 277760 | | |

 Disclaimer Statement:

 1. Do not scale off this Map.

 2. This map and any information supplied with it is furnished as a general guide, is only valid at the date of issue and no warrantly as to its correctness is given or implied. In particular this Map and any information suppoint on turns not be relied upon in the event of any development or works (including but not limited to excavations) in the vicinity of Severn Trent Water's assets or for the purposes of determining the suitability of a point of connection to the severage or distribution systems.

 3. On 1 October 2011 cost private severs and public severs and public lateral drains. A further transfer takes place on 1 October 2012 (date to be confirmed). Private pumping satians, which from part of these severs or lateral drains. A further transfer takes place on 1 October 2012 (date to be confirmed). Private pumping satians, which from part of these severs or lateral drains, Market transfer takes place on 1 October 2012 (date to be confirmed). Private pumping satians, which from part of these severs or lateral drains, Multi ansfer to the ownership of Severn Trent Water on or before 1 October 2016. Severn Trent Water dues not posses complete records of these assets. These assets may not be displayed on this Map.

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| Sewer Node Sewer Pipe Data | | | | | | | | | | |
|----------------------------|-------------|--------------------|----------------------|------|------|-------|-------------|-------------|----------|--------------|
| REFERENCE | COVER LEVEL | INV LEVEL UPSTR | INV LEVEL DOWNSTR | PURP | MATL | SHAPE | MAX SIZE | MIN SIZE | GRADIENT | YEAR LAID |
| SP34778201 | 71.37 | 67.70 | 67.60 | с | со | С | 1125 | nil | 276.60 | nill |
| SP34778203 | 70.79 | 67.86 | 67.70 | с | nil | С | 900 | nil | 92.94 | nill |
| SP34778204 | nil | nil | 67.86 | с | CI | С | 900 | nil | 0.00 | nill |
| SP34778205 | 72.26 | 69.35 | nil | с | CI | С | 900 | nil | 0.00 | nill |
| SP34778301 | 76.84 | 69.57 | 69.60 | с | со | E | 1520 | 1020 | 0.00 | nill |
| SP34778501 | 84.83 | 69.99 | 69.56 | с | со | E | 1520 | 1020 | 534.56 | nill |
| SP34779201 | 74.17 | 69.59 | 69.60 | с | BR | E | 1635 | 1020 | 0.00 | nill |
| SP34779202 | 74.05 | 69.50 | 69.35 | с | со | С | 900 | nil | 211.87 | nill |

| ★ X X Abandoned Gravity Sewer |
|---|
| Private Combined Gravity Sewer |
| Private Foul Gravity Sewer |
| Private Surface Water Gravity Sewer |
| Public Combined Gravity Sewer |
| Public Foul Gravity Sewer |
| Public Surface Water Gravity Sewer |
| Trunk Combined Gravity Sewer |
| Trunk Foul Use Gravity Sewer |
| Trunk Surface Water Gravity Sewer |
| Combined Use Pressurised Sewer |
| Foul Use Pressurised Sewer |
| └── → ── → Surface Water Pressurised Sewer |
| 🛏 🛁 — Highway Drain |
| Combined Lateral Drain (SS) |
| Foul Lateral Drain (SS) |
| |
| All Private Sewers are shown in magenta All section 104 sewers are shown in green All Sewers that have been transferred to Severn Trent Water after the 1 ^{et} October 2011, but have not been surveyed and |

| sewer | | Cable, Earthing |
|-----------|---|--|
| | > | Cable Junction |
| ity Sewer | | Cable, Optical Fibre/Instrumentation |
| ewer | | Cable, Low Voltage |
| tv Sewer | | Cable, High Voltage |
| ewer | +++++++++++++++++++++++++++++++++++++++ | Cable, Other |
| ver | B | Housing, Building |
| y Sewer | K | Housing, Kiosk |
| Sewer | DS. | Disposal Site |
| Sewer | STW | Sewage Treatment Works |
| S) | | Housing, Other |
| (0.0) | | Pipe Support Structure |
| n (SS) | | Sewage Pumping Facility |
| ent | \times | Sewer Facility Connection Inlet / Outlet |

| ۲ | Blind Shaft |
|---|----------------------|
| | Combined Use Manhole |

| \bigcirc | Flushing Chamber |
|------------|------------------|
| • | Foul Use Manhole |
| • | Grease Trap |
| | Head Node |
| - | Hydrobrake |
| | Lamphole |
| - | Outfall |
| | Overflow |
| - | Penstock |

Petrol Interceptor

| - | Sewer Chemical Injection Point | MATERIALS | CATEGORIES | |
|--------------|--|---|------------------------------|---|
| ÷. | Sewer Junction | NONE AC - ASBESTOS CEMENT | | |
| • | Sewerage Air Valve | BR - BRICK CC - CONCRETE BOX CULVERT | | |
|) ((| Sewerage Hatch Box Point | CO - CONCRETE CSB - CONCRETE SEGMENTS (BOLTED) | | |
| - | Sewerage Isolation Valve | CSU - CONCRETE SEGMENTS (UNBOLTED) DI - DUCTILE IRON | | |
| ~ | Soakaway | GRC - GLASS REINFORCED CONCRETE GRP - GLASS REINFORCED PLASTIC MAC - MASONRY IN REGULAR COURSES | N A | O/S Map scale: |
| 0 | Surface Water Manhole | MAR - MASONRY RANDOMLY COURSED PE - POLYETHLENE | W | Data of issue |
| | Vent Column | PF - POLYPROPYLENE PSC - PLASTIC STEEL COMPOSITE | \checkmark | Date of issue: |
| | Waste Water Storage | PVC - POLYVINYL CHLORIDE RPM - REINFORCED PLASTIC MATRIX | S | Sheet No. |
| ++++ | Pre-1937 Properties | SI - SPUN (GREY) IRON ST - STEEL U - UNKNOWN | W - WEIR C - CASCADE | Disclaimer Statements |
| | | VC - VITRIFIED CLAY | SE - SIDE ENTRY | 1. Do not scale off this Ma |
| | TABULAR KEY | | FV - FLAP VALVE | 2. This map and any information |
| Δ. | Sewer pipe data refers to downstream | SHAPE PURPOSE | BD - BACK DROP S - SIPHON | the purposes of determining the |
| | sewer pipe. | C - CIRCULAR C - COMBINED | HD - HIGHWAY DRAIN | 3. On 1 October 2011 most p |
| в. | Where the node bifurcates (splits) X and Y | E - EGG SHAPED E - FINAL EFFLUENT | S104 - SECTION 104 | ownership of Severn Trent Water Private pumping stations, which f |
| | indicates downstream sewer pipe. | R - RECTANGLE L - SLUDGE | | Severn Trent Water does not pos |
| с. | Gradient is stated a 1 in | S - SQUARE S - SURFACE WATER T - TRAPEZOIDAL U - UNKNOWN | | Reproduction by permission of Document users other than Sever should be made from it. |

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| 2. This map and any information suppl |
| and any information shown on it must |
| the purposes of determining the suitab |
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| ownership of Severn Trent Water and |
| Private pumping stations, which form p |
| Severn Trent Water does not possess |
| These assets may not be displayed |
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| Document users other than Severn Tre |
| should be made from it. |
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| SEVERN | | Severn Trent Wat Asset Data Mana | ter Limited gement | | | | |
|--------|------------------------|-------------------------------------|-----------------------|--|--|--|--|
| TRENT | | PO Box 5344 | | | | | |
| WATER | | Coventry CV3 9FT | | | | | |
| | | Telephone: 0845 601 6616 | | | | | |
| SE | SEWER RECORD (Tabular) | | | | | | |
| 1:17 | 50 | This map is centred upon: | | | | | |
| 28.0 | 1.19 | 0 / S Grid re | ference: | | | | |
| 1 of 1 | | х: | 434916 | | | | |
| | | у: | 277368 | | | | |

plied with it is furnished as a general guide, is only valid at the date of issue and no warranty as to its correctness is given or implied. In particular this Map t not be relied upon in the event of any development or works (including but not limited to excavations) in the vicinity of Severn Trent Water's assets or for biblity of a point of connection to the severage or distribution systems. The severs and private lateral drains in Severn Trent Water's assertage area, which were connected to a public sever as at 1.July 2011, transferred to the d became public severs and public lateral drains. A Unither transfer takes place on 1 Colober 2012 (date to be confirmed), part of these severs and public lateral drains, will transfer to the ownership of Severn Trent Water or or before 1 October 2016. Is complete records of these assets. d on this Map. Transce Survey on behalf of HMSO. © Crown Copyright and database right 2004. All rights reserved. Ordnance Survey licence number **100018202.** Trent Water business users are advised that this document is provided for reference purpose only and is subject to copyright, therefore, no further copies

APPENDIX 3

CCTV Sewer Survey Report



Wardell Armstrong LLP

OnSite Central Limited Worcester

| S | OnSite Central Limited Worcester Worcestershire WR3 8TJ Tel: 01905 340054, Fax: 01905 751571 | | | | | | | | | | |
|--|---|-----------|--------------|----------------|--------------|--------------|-----------|-------|-------|-------|--|
| | | | | Σ | Ø | | | | | | |
| Project name: Contract number: Contact: Date: Coventry 1901617 Andrew Smith 03/05/2019 | | | | | | | | | | | |
| No. | start MH | end MH | Date | Road | | Tape No. | Mat | erial | m | (m) | |
| 1 | SP34777701 | SP3477850 | 1 18/04/2019 | OFF BAR RO | AD | 00001 | Br | ick | 182.3 | 182.3 | |
| 2 | SP34778501 | SP3477830 | 1 18/04/2019 | OFF BAR RO | AD | 00001 | Br | ick | 229.9 | 17.8 | |
| 3 SP34777701 UPSTREAM 18/04/2019 OFF BAR ROAD | | | | | | 00001 | Br | ick | 20 | 12.2 | |
| 4 | SP34778301 | SP3477830 | 1 18/04/2019 | OFF BAR RO | AD | 00001 | Br | ick | 229.9 | 6.5 | |
| | | | Profile: | EGG SHAPED 150 | 0/1020 = 662 | 2.1 m (218.8 | <u>m)</u> | | | | |
| | | | | | | | | | | | |

all sections = 662.1 m (218.8 m)



OnSite Central Limited Worcester Worcestershire WR3 8TJ

| | | | | Inspecti | on report | | <u>, 1 ux.</u> | 0100010101 | |
|----------|---|------------------|--------------------------------|---|-----------------------------|--------------------------------------|---------------------|----------------|-------------|
| | Date: 18/04/2019 | J OSC | lob N°: C1901607 | Weather: No rain or snow | Operator: ONSITE BG | section num | ber: | PLI SP3477 | R: 7701X |
| , | Video Volume No.: Flow Ctrl: 00001 No flow control | | Temperature: above freezing | Year laid: Z | Cleaned: No | : | Strat. I not kr | Drain: nown | |
| Ro | ad: OFF B | AR ROAD | | Div/Dist: | | start MH: | SP34777 | 701 | |
| Pla | ce: COVE | NTRY | | Drain. Area: | | end MH: | SP34778 | 501 | |
| Lo | cation: Difficu | It access | | Standard: BS | EN 13508-2:2003 | Total length: | 182.3 m | | |
| Pu - | rpose: | Other | | | Shape/Size: Material: | Egg shaped 1500 Brick Pipe length | 0 /1020 : | | |
| l ly | e. | Gravity c | arain/sewe | r | Lining: | | | | |
| Co | mment: | | | | Linnig Type. | | | | |
| | | | | | | | | | |
| | 1:1350 | position | code | observation | | | MPEG | photo | grade |
| | | | | | | | | | |
| | SP3477701 | 0.00 | MH | Start node type, manhole, refere | ence number: SP34777701 | | 00:00:00 | | (Misc) 0 |
| | | 0.00 | WL | Water level 5 % height/diamete | r | | 00:00:00 | | (Serv) 0 |
| | | <u>0.00</u> S1 | DEE | Attached deposits encrustation | from 07 to 11 o'clock 5 % o | cross-sectional | 00:00:00 | | |
| | | <u>0.00</u> S2 | 2 DEE | Attached deposits encrustation | from 02 to 05 o´clock 5 % c | ross-sectional | 00:00:00 | | |
| | Ð | 20.00 | REM | General remark, Remark: CON | CRETE SOFFIT | | 00:02:55 | | |
| | | 28.00 | REM | General remark, Remark: BRIC | K SOFFIT | | 00:03:26 | | |
| | | 30.90 | CN | Connection at 12 o´clock, dia 10 | 00 mm | | 00:03:41 | | (Constr) 0 |
| % | | | | | | | | | |
| | | 126.50 | WL | Water level 10 % height/diamet | er | | 00:16:35 | | (Serv) 0 |
| | | <u>182.30</u> F1 | I DEE | Attached deposits encrustation area loss, Finish | from 07 to 11 o'clock 05 % | cross-sectional | 00:32:14 | | |
| | SD34779504 | 182.30 F2 | DEE | Attached deposits encrustation area loss, Finish | trom 02 to 05 o clock 05 % | cross-sectional | 00:32:14 | | |
| | Sr 04/1030 | 182.30 | MHF | ⊢ınısh node type, manhole, refe | rence number: SP3477850 | I | 00:32:14 | | |
| Str | uctural Defects | | | | Constructional Features | | | | |
| Ser | vice Defects | | | | Miscellaneous Features | | | | |





OnSite Central Limited Worcester Worcestershire WR3 8TJ el: 01905 340054. Fax: 01905 751571

| | | | | | | | Inspecti | on report | | | | |
|---------------------------------|-------------------|------------------|--------|------------|------------------|---------------|---|--------------------------|-------------------|--------------|--------------|--------------|
| | Date: 18/04/20 | 19 | | Job | o N°: 1901607 | | Weather: No rain or snow | Operator: ONSITE BG | section nu | mber: | PL SP3477 | R: /8501X |
| V | /ideo Volum | e No.: | | Flov | v Ctrl: | 1 | Temperature: | Year laid: | Cleane | aned: Strat. | | Drain: |
| | | | | o now | contro | | | Σ | NO | | | IOWN |
| Roa | ad: | | | | | | Div/Dist: | | start MH: | SP34778 | 301 | |
| Loc | ation: | Difficult | access | | | | Standard: BS | EN 13508-2:2003 | Total length: | 229.9 m | 501 | |
| Pur | pose: | | Other | r | | | | Shape/Size: | Egg shaped 15 | 00/1020 | | |
| Тур | e: | | Gravi | ity dra | ain/sewe | r | | Material: | Brick Pipe leng | th: | | |
| Use | e: | | Com | bined | | | | Lining Type: | | | | |
| Cor | nment: | | | | | | | | | | | |
| 1:150 position code observation | | | | | | ervation | | | MPEG | photo | grade | |
| | SP347785 | 01 | 0.00 | | МН | Star | : node type, manhole, refere | ence number: SP3477850 | 01 | 00:00:00 | | (Misc) 0 |
| | | | 0.00 | | WL | Wat | er level 05 % height/diamet | er | | 00:00:00 | | (Serv) 0 |
| | | | 0.00 | S1 | DEE | Atta | ched deposits encrustation | from 07 to 11 o´clock 05 | % cross-sectional | 00:00:00 | | |
| | | | 0.00 | S 2 | DEE | area Atta | loss, Start ched deposits encrustation | from 02 to 05 o´clock 05 | % cross-sectional | 00:00:00 | | |
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| | | | 16.80 | | \ \ /I | Wat | er level 20 % height/diamet | or | | 00.05.52 | | (Sen/) 0 |
| | | | 17.50 | | | vval | | | | 00.00.02 | | |
| | - | | 17.50 | | VVL | vvat | er ievel 10 % neight/diamet | | | 00:06:37 | | (Serv) 0 |
| | · · · · · · | \mathbb{N}^{-} | 17.80 | F1 | DEE | Attao area | ched deposits encrustation loss, Finish | trom 07 to 11 o´clock 05 | % cross-sectional | 00:31:20 | | |
| | | | 17.80 | F2 | DEE | Attao area | ched deposits encrustation loss, Finish | from 02 to 05 o´clock 05 | % cross-sectional | 00:31:20 | | |
| | | \backslash | 17.80 | | SA | Surv | ey abandoned, Remark: Dl | JE TO LOSS OF TRACT | ION | 00:31:20 | | (Misc) 0 |
| | | | | | | | | | | | | |
| Stru | ctural Defect | is | | | | | | Constructional Features | | | | |
| Serv | vice Defects | | | | | | | Miscellaneous Features | | | | |





OnSite Central Limited Worcester Worcestershire WR3 8TJ

| | | | | | Inspect | ic | on report | 161.0 | 1903 340034, 1 ax. | . 01903 73137 | 1 |
|------------------------------------|--|-----------|---------------------|--------------------------------|---|------------|---|--------------------------------|--------------------------------|---------------|-------------|
| Date 18/04/2 | : 019 | J | lob N°: C1901607 | | - Weather: No rain or snow | | Operator: ONSITE BG | section n | umber: | PL UPSTR | R: EAM X |
| Video Volui 0000 | Video Volume No.: Flow Ctrl: 00001 No flow control | | | Temperature: above freezing | | Year laid: | Clear No | ied: | ed: Strat. Drain: not known | | |
| Road: | OFF BAR | ROAD | | | Div/Dist: | | | start MH: | SP34777 | 7701 | |
| Place: | COVENT | RY | | | Drain. Area: | | | end MH: | UPSTRE | AM | |
| Location: | Difficult a | ICCESS | | | Standard: BS | S E | N 13508-2:2003 | Total length: | 20 m | | |
| Purpose: | | Other | | | | | Shape/Size: Material: | Egg shaped 1 Brick Pipe len | 500/1020 ath: | | |
| Type: | | Gravity d | Irain/sewe | r | | | Lining: | | 3 | | |
| Comment: | | Combine | a | | | | Lining Type: | | | | |
| Comment. | | | | | | | | | | | |
| 1:10 | 0 po | osition | code | obse | ervation | | | | MPEG | photo | grade |
| | | 0.00 | МН | Start | node type manhole refe | rei | nce number: SP34777701 | | 00:00:00 | | (Misc) 0 |
| SP34777 | | 0.00 | \\\/I | Mat | er level 0 % height/diamet | | | | 00.00.00 | | (Sen/) 0 |
| | \backslash | 0.00 | | Gon | aral romark. Romark: CON | | | | 00:00:00 | | (0010)0 |
| | \sim | 0.00 | | Gen | erar remark, Remark. COI | NC. | | | 00.00.00 | | |
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| - | | 7.70 | LL | Line | of drain/sewer deviates le | ft | | | 00:00:56 | | |
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| | = | 12.20 | SA | Surv | ey abandoned, Remark: F | ю | INT OF REQUIRED SURV | ΈY | 00:01:32 | | (Misc) 0 |
| | | | | | | | | | | | |
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| Structural Defe Service Defects | CTS | | | | | \neg | Constructional Features Miscellaneous Features | | | | |
| | | | | | | | | | | | |



OnSite Central Limited Worcester Worcestershire WR3 8TJ

| | | | | | Inspec | ctic | on report | | | | |
|-------------------|--|-------------|---------------------|--------------------------------|--|-----------------|---|------------------------------------|-----------------------|----------------|--------------|
| Dat 18/04/ | te: /2019 | osc | Job N°: C1901607 | | Weather: No rain or snow | | Operator: ONSITE BG | section nui | mber: | PL SP3477 | R: '8301X |
| Video Volu 000 | Volume No.: Flow Ctrl: 00001 No flow control | | | Temperature: above freezing | | Year laid: Clea | | d: | Strat. I not kr | Drain: 10wn | |
| Road: | OFF BAI | ROAD | | Div | /Dist: | | | start MH: | SP34778 | 301 | |
| Place: | COVENT | RY | | Dra | in. Area: | | | end MH: | SP34778 | 301 | |
| Location: | Difficult | access | | Sta | ndard: | BS E | N 13508-2:2003 | Total length: | 229.9 m | | |
| Purpose: | | Other | | | | | Shape/Size: Material: | Egg shaped 150 Brick Pipe lengt | 00/1020 th: | | |
| Type: | | Gravity (| drain/sewe | • | | | Lining: | | | | |
| Comment: | | Combine | eu | | | | Lining Type: | | | | |
| 1:5 | 50 r | osition | code | observati | on | | | | MPEG | photo | grade |
| SP3477 | 8301 | 0.00 | МН | Start node | e type, manhole, r | eferer | nce number: SP347783 | 01 | 00:00:00 | | (Misc) 0 |
| | | 0.00 | WL | Water lev | el 10 % height/dia | amete | r | | 00:00:00 | | (Serv) 0 |
| | | 1.10 | MM | Missing m | ortar at 03 o´cloc | k | | | 00:00:22 | | (Struct) 2 |
| _ | | 2.00 | REM | General re | emark, Remark: [| DEBR | IS UNDERWATER | | 00:01:00 | | |
| | | <u>6.50</u> | DER | Settled de UNDER C | posits coarse 20 RAWLER andoned. Remar | % crc | ss-sectional area loss, E TO LOSS OF TRACT | Remark: BUILT UP | 00:04:05 00:04:06 | | (Misc) 0 |
| | | 0.30 | 5A | Surveyab | andoned, Remai | K. DU | | | 00.04.00 | | (MISC) U |
| Structural Def | fects | | | | | | Constructional Features | | | | |
| Service Defec | ts | | | | | | Miscellaneous Features | | | | |

Access Paths to Site Via Shortly Road & Whitley Depot





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Manhole 7701 - Upstream



Channel upstream of MH7701 is dry with no incoming flows

Manhole 7701 - 8501



Shallow depth of water downstream of manhole 7701

Manhole 7701 - 8501



Water increases in depth with distance downstream of 7701. Standing water assumed to be caused partially by incoming warm air from a laundrette downstream of the site within the industrial estate

Manhole 8501 to 8301



Silt and standing water downstream of manhole 8501

APPENDIX 4

Lead Local Flood Authority Correspondence

Griffiths, Bryn

| From: | Speed, Harvey <harvey.speed@coventry.gov.uk></harvey.speed@coventry.gov.uk> |
|--------------|---|
| Sent: | 16 January 2019 17:15 |
| То: | Griffiths, Bryn |
| Cc: | Flooding |
| Subject: | RE: Flood Risk Assessment Pre-Application Advice |
| Attachments: | FRM & Drainage Standing Advice.pdf |
| | |

Categories: Saved

Good afternoon Ben,

Thank you for your email. We would be happy to support you with this important development for the city.

As the site has historically been for the use of allotments, we have not been made aware of any flooding issues here.

I have attached our standing advice for developers which should give you a solid foundation for developing a robust FRA for the site.

Please let me know if I can be of any further assistance.

Kind regards,

Harvey

Harvey Speed Drainage & Flood Risk Engineer Coventry City Council Whitley Depot, 259 London Road Coventry, CV3 4AR E: <u>harvey.speed@coventry.gov.uk</u> T: 02476 834314 M: 07552 277026

From: Griffiths, Bryn <<u>bgriffiths@wardell-armstrong.com</u>>
Sent: 08 January 2019 15:51
To: Flooding <<u>Flooding@coventry.gov.uk</u>>
Subject: Flood Risk Assessment Pre-Application Advice

Mimecast Attachment Protection has deemed this file to be safe, but always exercise caution when opening files.

Good afternoon,

I am producing a Flood Risk Assessment and Surface Water Drainage Strategy in support of outline planning for a proposed Materials Recovery Facility on land off Shortley Road, Coventry, CV3 4AE. As part of the assessment I was looking to get planning advice off yourselves as the Lead Local Flood Authority. Having looked at the Coventry City Council Lead Local Flood Authority Pre-Application Charging Scheme document, it would seem the information we would need to provide to receive pre-application advice is a bit too extensive for what we would have at this stage of the project as it is your advice (along with that of the EA and Severn Trent Water) which we would use to produce the Flood Risk Assessment, discharge calculations and drainage design. There is currently no

Is there a way of obtaining advice such as historic flood incidents or drainage problems with respect to ordinary watercourses, surface water and groundwater and also advice on local guidance and policies relating to surface water management/SuDS in this area, which could then be used to assist our assessment?

Many thanks, Bryn

Bryn Griffiths | Senior Environmental Scientist Wardell Armstrong LLP Sir Henry Doulton House, Forge Lane, Etruria, Staffordshire, UK t: 01782 276700 m: 07469 856653



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Coventry City Council

Flood Risk Management & Drainage – Planning Standing Advice

Policy

Coventry City Council as Lead Local Flood Authority (LLFA) has the responsibility of managing local surface water and groundwater flood risk within its area. The LLFA is also a statutory consultee on planning applications within certain specific criteria, ensuring the delivery of sustainable development. The Council has produced a number of documents relating to flood risk and surface water management and contributed to the emerging Coventry Local Plan with the Strategic Flood Risk Assessment as a Supplementary Planning Document. The documents listed below can be found online at http://www.coventry.gov.uk/watermanagementandflooding.

- Preliminary Flood Risk Assessment (PFRA)
- Surface Water Management Plan (SWMP)
- Local Flood Risk Management Strategy (LFRMS)
- Strategic Flood Risk Assessment (SFRA)

Flood risk mapping

The Environment Agency's fluvial, pluvial and reservoir flood risk mapping for individual properties in England can be found at <u>https://www.gov.uk/check-flood-risk</u>.

Surface water discharge

In accordance with Building Regulations 2010, Part H, rainwater shall discharge to one of the following, in order of priority; (i) an adequate infiltration system; (ii) a watercourse or; (iii) a sewer. Where a development cannot infiltrate, <u>runoff should be restricted to the Qbar greenfield</u> <u>discharge rate minus 20%, or 5l/s</u>, whichever is greater.

Climate Change

The Environment Agency revised its climate change allowances in February 2016 and these can be located at <u>https://www.gov.uk/guidance/flood-risk-assessments-climate-change-allowances</u>. Based on this data, the LLFA requires all development to apply a <u>40% climate change allowance</u> to surface water drainage design, unless justification for a lower allowance can be made, strictly in accordance with the national criteria.

Adoption and maintenance

The adoption and maintenance of all drainage features are a key consideration to ensure the long term operation at the designed standards. Underground drainage infrastructure such as pipes and tanks will be considered for adoption by a Sewerage Undertaker. The Council will consider the adoption of open air sustainable drainage within areas of public open space, such as semi-dry detention basins, forming joint-use amenity space.

Open attenuation features must be accessible by appropriate maintenance vehicles and require a maintenance strip with a minimum width of 3 metres. The maintenance strip adjoining a watercourse should be a minimum width of 5 metres. However, these figures are only a minimum guide and wider strips may be required to suit the specific development.

Exceedance flows

Developers should demonstrate flood flow routes through a site in the event of design exceedance or blockage scenario, and where the site topography naturally channels through the site prior to development. Flood flows should be managed to be safe and not enter any buildings

or disrupt emergency access routes. Exceedance flows should not be routed through private residential plots. If flooding occurs, the hazard should be considered in line with the latest guidance from the Environment Agency.

Prevention of flows onto the Public Highway

Developers should ensure that surface water is managed to prevent flows onto the Public Highway. Private parking and hardstanding with a gradient falling towards the Public Highway should ensure that adequate private drainage is provided to intercept surface water flows.

Water quality

The Water Framework Directive (WFD) established a legal framework for the protection, improvement and sustainable use of water bodies such as rivers, brooks and groundwater. Developers are required to contribute to the aims of the WFD, which broadly involve:

- prevent deterioration in water body status;
- reduce water pollution;
- conserve aquatic ecosystem and habitats;
- reduce the effects of floods and droughts on water bodies;
- promote sustainable use of water as a natural resource; and
- removal of physical man-made modifications to watercourses, i.e. culverts.

Useful guidance and design standards

A range of drainage design guidance documents and British Standards are available. Some examples are listed below, but is not exhaustive:

| The Building Regulations, Part H | Free download from Planning Portal |
|---|---|
| National Planning Policy Framework | Free to access at GOV.UK |
| Planning Practice Guidance | Free to access at GOV.UK |
| The SuDS Manual (C753) | Free download from CIRIA |
| Planning for SuDS – making it happen (C687) | Free download from CIRIA |
| Designing for exceedance in urban drainage – | Free download from CIRIA |
| good practice (C635) | |
| Assessing and managing flood risk in | Purchase from BSI |
| development – Code of Practice | |
| (BS 8533:2011) | |
| Drain and sewer systems outside buildings | Purchase from BSI |
| (BS EN 752:2008) | |
| Design Manual for Roads and Bridges: Volume | Free downloads available from Standards for |
| 4 Geotechnics and Drainage | <u>Highways</u> |
| Sewers for Adoption: A design and | Purchase from WRC |
| construction guide for developers (7 th Edition) | |
| BRE Digest 365 – Soakaway Design | Purchase from BRE |

Detailed pre-application advice and meetings

More detailed and site-specific flood risk management and drainage advice can be sought by submitting a request for pre-application advice to the LLFA. Further details on the LLFA pre-application charging scheme can be found online. A summary of the costs for this service are as follows:

| Category A – Small Scale Development | £60 | |
|---------------------------------------|-------|-----------|
| Category B – Small Scale Development | £120 | |
| Category C – Medium Scale Development | £600 | |
| Category D – Large Scale Development | £1200 | |
| Category E – Major Scale Development | £1800 | |
| Category F – Project/Major Work | £2400 | (minimum) |

APPENDIX 5

Environment Agency Correspondence

Griffiths, Bryn

| _ | |
|-------------|--|
| From: | Enquiries_Westmids <enquiries_westmids@environment-agency.gov.uk></enquiries_westmids@environment-agency.gov.uk> |
| Sent: | 31 January 2019 14:12 |
| То: | Griffiths, Bryn |
| Subject: | Our Ref 112369 Flood Level Data and Planning Advice - Bar Road, Coventry |
| Categories: | Saved |



Our Ref: 112369

Your Ref:

Date: 31 January 2019

Dear Bryn

Enquiry regarding: flood level data and planning advice – Bar Road, Coventry

Thank you for your enquiry which was received on 10 January.

We respond to requests under the Freedom of Information Act 2000 and Environmental Information Regulations 2004.

There is no historic flooding in this area. We have no planning advice to offer regarding flood risk as the site is located within Flood Zone 1.

We would advise you to contact the LLFA (local lead flood authority) for guidance on your drainage strategy.

Their details are

Email: flooding@coventry.gov.uk

Tel: 08085 834333

Whitley Depot 259 London Road Coventry CV3 4AR Please refer to Open Government Licence which explains the permitted use of this information.

Please get in touch if you have any further queries or contact us within two months if you'd like us to review the information we have sent.

Yours sincerely

Diane Edwards Customers & Engagement Officer West Midlands Area

For further information please contact the Customers & Engagement team on Tel. 02084 747856 E-mail:- Enquiries_WestMids@environment-agency.gov.uk

From: Griffiths, Bryn [mailto:bgriffiths@wardell-armstrong.com]
Sent: 08 January 2019 15:06
To: Enquiries, Unit <<u>enquiries@environment-agency.gov.uk</u>>
Subject: Flood Level Data and Planning Advice - Bar Road, Coventry

Good afternoon,

I am preparing a Flood Risk Assessment and Drainage Strategy for a proposed c.3.6 hectare Materials Recovery Facility off Bar Road, Coventry (grid reference 434812, 277541). As shown on the attached Flood Map for Planning, the site is adjacent to areas of Flood Zone 3 associated with the River Sherbourne. In order to assist me, I would like to request any flood level data you hold for this watercourse.

To assist me further with the drainage strategy, I would welcome any pre-planning comments that you wish to make with respect to flood risk and drainage at the site. In particular, if you have any local knowledge of historic flood incidents or drainage problems with respect to ordinary watercourses, surface water and groundwater, I would be grateful if you could comment. I would also be grateful for any advice on local guidance and policies relating to surface water management/SuDS in this area.

Please let me know if I can provide further information to assist you.

Many thanks, Bryn

Bryn Griffiths | Senior Environmental Scientist Wardell Armstrong LLP Sir Henry Doulton House, Forge Lane, Etruria, Staffordshire, UK t: 01782 276700 m: 07469 856653





APPENDIX 6 Greenfield Runoff Calculations

Greenfield runoff rate estimation tool



Calculated by:

be

Page 1 of 1

estimation for sites

www.uksuds.com | Greenfield runoff tool

Site Details

B Griffiths Site name: Coventry MRF Latitude: 52.39476° N Site location: London Road, Coventry Longitude: 1.48985° W This is an estimation of the greenfield runoff rates that are used to meet normal best practice criteria in line with Environment Agency guidance "Rainfall runoff management Reference: 2981626558 for developments", SC030219 (2013), the SuDS Manual C753 (Ciria, 2015) and the non-statutory standards for SuDS (Defra, 2015). This information on greenfield runoff rates may Date: Jul 21 2020 13:28 the basis for setting consents for the drainage of surface water runoff from sites. **Runoff estimation approach** IH124 Site characteristics Notes Total site area (ha): 3.31 (1) Is Q_{BAR} < 2.0 I/s/ha? Methodology When Q_{BAR} is < 2.0 I/s/ha then limiting discharge rates are set at Q_{BAR} estimation method: Calculate from SPR and SAAR 2.0 l/s/ha. SPR estimation method: Calculate from SOIL type Soil characteristics Default Edited (2) Are flow rates < 5.0 l/s? SOIL type: 4 4 HOST class: N/A N/A Where flow rates are less than 5.0 l/s consent for discharge is SPR/SPRHOST: usually set at 5.0 l/s if blockage from vegetation and other 0.47 0.47 materials is possible. Lower consent flow rates may be set where Hydrological characteristics the blockage risk is addressed by using appropriate drainage Default Edited elements. SAAR (mm): 655 655 (3) Is SPR/SPRHOST ≤ 0.3 ? Hydrological region: 4 4 Growth curve factor 1 year: Where groundwater levels are low enough the use of soakaways 0.83 0.83 to avoid discharge offsite would normally be preferred for Growth curve factor 30 years: 2 2 disposal of surface water runoff. Growth curve factor 100 years: 2.57 2.57 Growth curve factor 200 years: 3.04 3.04 Greenfield runoff rates

| | Default | Edited |
|-------------------------|---------|--------|
| Q _{BAR} (I/s): | 14.78 | 14.78 |
| 1 in 1 year (l/s): | 12.27 | 12.27 |
| 1 in 30 years (l/s): | 29.57 | 29.57 |
| 1 in 100 year (l/s): | 38 | 38 |
| 1 in 200 years (l/s): | 44.95 | 44.95 |

This report was produced using the greenfield runoff tool developed by HR Wallingford and available at www.uksuds.com. The use of this tool is subject to the UK SuDS terms and conditions and licence agreement, which can both be found at www.uksuds.com/terms-and-conditions.htm. The outputs from this tool are estimates of greenfield runoff rates. The use of these results is the responsibility of the users of this tool. No liability will be accepted by HR Wallingford, the Environment Agency, CEH, Hydrosolutions or any other organisation for the use of this data in the design or operational characteristics of any drainage scheme.

APPENDIX 7

Surface Water Attenuation Calculations

Calculation Sheet



REF:

| | | | CALC REE NO | | | |
|---|-----------------------------|---------------------------------|------------------------------|--|--|--|
| Coventry City Council | Coventry MRF | CA11485 | PAGE: 1 OF 2 | | | |
| ALCULATION | CALC. BY: | CHECKED BY: | APPROVED BY: | | | |
| Attenuation Calculation - | (NAME AND SIGNATURE) | (NAME AND SIGNATURE) | (NAME AND SIGNATURE) | | | |
| Development Plateau | B Griffiths | | | | | |
| | DATE: 21/07/2020 | DATE: | DATE: | | | |
| | | | | | | |
| Quick Storage Estimate | | | | | | |
| FSR Region | England & Wales Y | Estimate is based on no infil | tration to ground to provide | | | |
| M5-60 (mm) | 20.000 | a 'worst case' estimate | | | | |
| Ratio-R | 0.400 | Infiltration rate to be confirm | med by BRE-365 soakaway | | | |
| Summer CV 🗸 | 0.750 | testing | | | | |
| Winter CV | 0.840 | | | | | |
| 1 in 30 Year Storm Event | | | | | | |
| Storage Estimate | | | | | | |
| Return Period (years) | 30 | 1 | | | | |
| Climate Change (%) | 0 | | | | | |
| impermeable Area (ha) | 2.450 | | | | | |
| Peak Discharge (I/s) | 8 800 | 1 | | | | |
| Infiltration Coefficient (m/hr) | | 1 | | | | |
| (leave blank if no infiltration) | | | | | | |
| Required Storage (m ³) | Calc | | | | | |
| from | 812 | 1 | | | | |
| to | 1122 |] | | | | |
| | | | | | | |
| Approvimato storago | roquirod: 967m ³ | | | | | |
| Approximate storage | | | | | | |
| 1 in 100 Year + Climate Chan | ge Storm Event | | | | | |
| | | | | | | |
| Storage Estimate | | | | | | |
| Return Period (years) | 100 | | | | | |
| Climate Change (%) | 40 | | | | | |
| Impermeable Area (ha) | 2.450 | | | | | |
| Peak Discharge (I/s) | 8.800 | | | | | |
| Infiltration Coefficient (m/hr) (leave blank if no infiltration) | | | | | | |
| Required Storage (m ³) | Calc | | | | | |
| from | 1712 | | | | | |
| to | 2163 | | | | | |
| | | | | | | |
| Approximate storage i | equired: 1937m ³ | | | | | |
| | | | | | | |

Calculation Sheet



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| ALCULATION | CALC. BY: | | СНЕСКЕ | D BY: | | APPROVED BY: | | | |
| Attenuation Calculation - | (NAME AND SIGNATURE) | | (NAME AND SIGNATURE) | | (NAME AND SIGNATURE) | | | | |
| Final Outfall | B(| Griffiths | | | | | | | |
| | DATE: 2 | 21/07/2020 | DATE | : | | DATE: | | | |
| | | | | | | | | | |
| Quick Storage Estimate | | | | | | | | | |
| FSR Region | England & W | ales 🛩 | Estima | ate is based (| on no infilt | tration to | ground | to provide | |
| M5-60 (mm) | 20.000 | | a 'wor | st case' estir | nate | | | | |
| Ratio-R | 0.400 | | Infiltra | ation rate to | he confirm | ned by BR | F-365 s | oakaway | |
| Summer CV | 0.750 | | testing | g | | | L-303 3 | Jakaway | |
| Winter CV | 0.840 | | | | | | | | |
| 1 in 30 Year Storm Event | | | | | | | | | |
| | | | | | | | | | |
| Storage Estimate | | | | | | | | | |
| Return Period (years) | 30 | | | | | | | | |
| Climate Change (%) | 0 | | | | | | | | |
| Impermeable Area (ha) | 2.770 | | | | | | | | |
| Peak Discharge (I/s) | 10.000 | | | | | | | | |
| Infiltration Coefficient (m/hr) (leave blank if no infiltration) | | | | | | | | | |
| Required Storage (m ³) | Calc | | | | | | | | |
| from | 917 | | | | | | | | |
| to | 1267 | | | | | | | | |
| Approximate storage r | equired: 1,0 | 92m ³ | | | | | | | |
| 1 in 100 Year + Climate Chan | ge Storm Ev | ent | | | | | | | |
| Storage Estimate | | | | | | | | | |
| Return Period (years) | 100 | | | | | | | | |
| Climate Change (%) | 40 | | | | | | | | |
| Impermeable Area (ha) | 2.770 | | | | | | | | |
| Peak Discharge (I/s) | 10.000 | | | | | | | | |
| Infiltration Coefficient (m/hr) (leave blank if no infiltration) | | | | | | | | | |
| Required Storage (m ^s) | Calc | | | | | | | | |
| from | 1933 | | | | | | | | |
| to | 2444 | | | | | | | | |
| | | | | | | | | | |
| Approximate storage r | equired: 2,1 | 89m ³ | Add | ditional 252m | ° of storage | e required | in weigh | bridge | |
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NOTES

- 1. TO BE READ IN CONJUNCTION WITH FLOOR RISK ASSESSMENT (REF: CA11485-0005).
- 2. TOPOGRAPHICAL DATA FROM "DTM 1m" LIDAR MODEL.

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